

CALCULATOR PROGRAMS

1



HEWLETT PACKARD
PROGRAMMABLE CALCULATORS

STRUCTURAL PROGRAMS
FOR

HP41C

CALPRO 

CALCULATOR PROGRAMS

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These calculator programs are designed for the solution of specific Structural Engineering problems within the capabilities of the HP-41C Hewlett Packard calculators. It is most important that all data and program applications be supplemented by professional judgment of qualified engineering personnel. No responsibility is assumed by the writer for any errors, mistakes, misrepresentations that may occur when using these programs.

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Structural Programs for HP-41C

Hewlett Packard Programmable Calculators

HEWLETT PACKARD
PROGRAMMABLE CALCULATORS

STRUCTURAL PROGRAMS
FOR
HP41C

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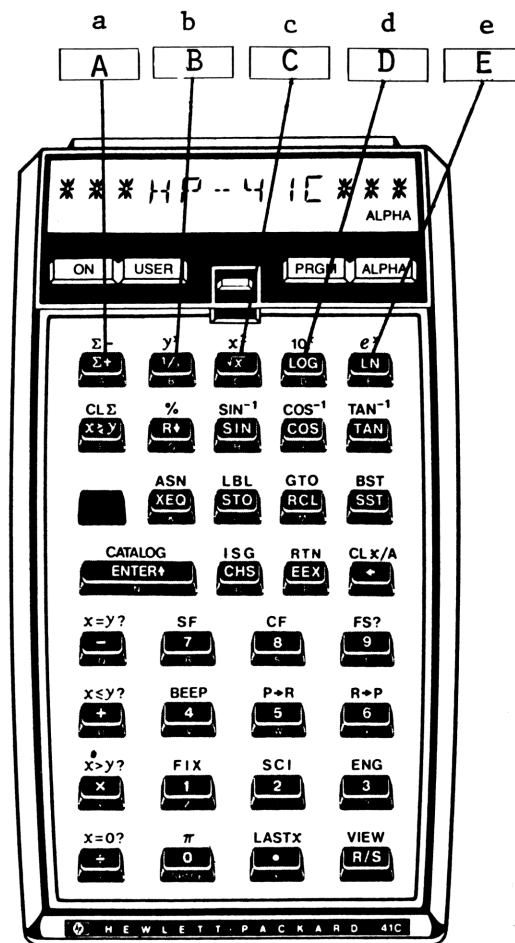


STRUCTURAL PROGRAMS

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KEY LAYOUT FOR USE WITH PROGRAMS



PROGRAM EXECUTION REQUIRES HP-41C CALCULATOR, CARD READER,
PRINTER AND TWO ADDITIONAL MEMORY MODULES

HANDY REFERENCE

Working Stress Design Versus Ultimate Strength Design in Programs

$$\text{Steel Percentage } P = \frac{P_b}{2}$$

Factor Safety or Load Factor = 1.7

USD

Beam Shear $v_u = \frac{V_u}{\phi b d} = 2 \sqrt{f'c}$ $v(\text{allow}) = \frac{v_u}{1.7} \times \phi = \frac{V_u}{1.7 b d} = \frac{2 \phi \sqrt{f'c}}{1.7}$

WSD

$v(\text{allow}) = 55\% \times 2 \sqrt{f'c} = 1.1 \sqrt{f'c}$ $\phi = 0.85$

USD

Punching Shear $v_u = \frac{V_u}{\phi b d} = 4 \sqrt{f'c}$ $v(\text{allow}) = \frac{v_u}{1.7} \times \phi = \frac{V_u}{1.7 b d} = \frac{4 \phi \sqrt{f'c}}{1.7}$

WSD

$v(\text{allow}) = 50\% \times 4 \sqrt{f'c} = 2 \sqrt{f'c}$ $\phi = 0.85$

CONCRETE STRENGTH		f'c = 3000 PSI		f'c = 4000 PSI		f'c = 5000 PSI	
Factor	f _y	WSD	USD	WSD	USD	WSD	USD
a	40,000 PSI	1.44	$\frac{2.56}{1.7} = 1.51$	1.44	1.51	1.44	1.51
	60,000 PSI	1.76	$\frac{3.94}{1.7} = 2.32$	1.76	2.32	1.76	2.32
K	40,000 PSI	226	$\frac{575}{1.7} = 338$	324	$\frac{767}{1.7} = 451$	426	$\frac{895}{1.7} = 526$
	60,000 PSI	204	$\frac{497}{1.7} = 292$	295	$\frac{662}{1.7} = 389$	390	$\frac{794}{1.7} = 467$
Beam v _c		60 PSI	$\frac{110}{2} = 55 \text{ PSI}$	70 PSI	$\frac{126}{2} = 63 \text{ PSI}$	78 PSI	$\frac{141}{2} = 71 \text{ PSI}$
Footing v _o		110 PSI	$\frac{220}{2} = 110 \text{ PSI}$	126 PSI	126 PSI	141 PSI	141 PSI

RETAINING WALLS "RETWALL" PROGRAM DESCRIPTION

Program divides stem into 10 equal parts and calculates the moment, section thickness, required and minimum reinforcement for a vertical wall of constant or uniformly varying thickness with a horizontal backfill with or without a surcharge loading; stem shear; overturning moment; base shear; factor safety; weight; resultant location; toe and heel pressures, shear and reinforcement; passive pressure and key depth if required. Will also handle concentrated loads and moments on walls. Uses Working Stress or Ultimate Strength Design divided by the appropriate load factor per ACI 318-77.

This program uses Working Stress Design (Alternate Design Method) or Ultimate Strength Design (Load Factor Design) when divided by the load factor of 1.7 to determine the reinforcement and moment capacities. Remember that both "K" and "a" must be divided by the appropriate load factor prior to entry into the calculator. Sheet iii-1 has been included in the front of this manual for ready reference of WSD and USD values for "a", "K", " v_o " and " v_c " for 3000 PSI, 4000 PSI, and 5000 PSI concrete.

The footing or wall thickness is not increased internally, so the actual moments and shears must be compared with the allowables to insure that footing and wall can resist the loads with an adequate factor of safety. The program will not design for compressive reinforcement so the actual moments should never exceed the allowable moments.

The program will automatically use the surcharge loading to calculate the stem moments, shears, reinforcement, overturning moment, etc. but will not include the surcharge in the resisting moment or total wall weight unless Label D is pushed (sets flag zero). Most references suggest, however, that the surcharge not be included in the wall weight or resisting moment, which means that you could normally forget Label D.

The CRSI Design Handbook or other sources can be used to obtain preliminary design parameters for use in the wall and footing.

If the footing that you input produces a heel pressure less than zero at the tip of the heel, the program will automatically increment the toe and heel projections as required until a positive pressure exists on tip of footing heel. You as the designer can increment both heel and toe projections, hold the toe projection or hold the heel projection. Do this as follows:

Increment both heel (HP) and toe (TP) projections

Push Label A (This clears flags 1 and 2)
Then enter your data as shown on the calculation sheet

Hold toe (TP) projection

Push Label B (This sets flag 1 and clears flag 2)
Then enter your data as shown on the calculation sheet

Hold heel (HP) projection

Push Label C (This sets flag 2 and clears flag 1)
Then enter your data as shown on the calculation sheet

The concentrated load on the retaining wall is not used by the program to increase or reduce the wall reinforcement. However, the concentrated load is used in determining the resisting moment and total wall weight and is therefore used to design the footing. The position (Z) of this load is determined from the toe and will be incremented internally when the toe projection is changed.

When you have a sloping wall, the triangular wedge of soil above the rear face of wall is treated as concrete by the program. The P and Z may be used to correct the resisting moment when the slope is large. It can normally be ignored for small slopes.

The design of the key is based upon obtaining a 1.5 safety factor against sliding. The resistance to sliding is based upon the coefficient of friction (0.45) times the total wall and soil weight (W) and the resistance from passive pressure. Passive pressure is calculated using 300 PSF/foot of depth.

$$1.5 \times \text{BASE SHEAR} = \text{Coef of Friction} \times W + 0.3 H^2/2$$

$$\text{Weight of concrete} = 150 \text{ LBS/FT}^3$$

$$\text{Weight of soil} = 100 \text{ LBS/FT}^3$$

H = Height of wall above footing (feet)

STMT = Stem thickness at top of wall (inches)

Dl = Centerline of reinforcement to back face of wall (inches)

D = Total slope of back face of stem (inches)

Soil H = Equivalent fluid pressure of soil on rear of wall (kips/ft³)

HS = Height of surcharge (feet)

a = $f_s j / 12,000$ for Working Stress Design

= $a_u / 1.7 = \phi f_y (1 - .59w) / (12,000 \times 1.7)$ for Ultimate Strength Design

K = $\frac{1}{2} f_c k j = M / F = M / b d^2$ for Working Stress Design

= $M_u / (F \times 1.7) = \phi f'_c w (1 - .59w) / 1.7 = K_u / 1.7$ for Ultimate Strength Design

M (allow) = KF (Kip FT)

v = Shear at bottom of stem, or face of stem for heel or toe

TP = Toe projection (feet)

HP = Heel projection (feet)

FTG = Footing thickness (feet)

M_O = Overturning moment (Kip-feet)

BASE SHEAR = Total horizontal force on rear of wall (kips)

SHT = Depth of soil on toe (feet)

FTG WIDTH = Total footing width (feet)

M_R = Resisting moment of wall, footing and earth about toe (Kip-feet)

W = Total weight of stem, footing and earth on heel and toe (kips)

$\bar{x}$ = Location of resultant from toe (feet)

f_p = Soil pressure on tips of heel or toe (Kips/FT²)

A_s = Area of reinforcement (square inches)

If you should desire a printout of the prompting statements along with the numerical results, set your printer to the normal mode.

The main program may be addressed directly by the execute key (XEQ) and the program name "RETWALL". Flag eleven has been set causing the program to initialize immediately upon entry of program cards.

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PUSH LABEL A, INCREMENTS HEEL AND TOE

PUSH LABEL B, HOLDS TOE PROJECTION (TP)

PUSH LABEL C, HOLDS HEEL PROJECTION (HP)

PUSH LABEL D IF SURCHARGE IS TO BE INCLUDED IN RESISTING MOMENT.

1

SAMPLE PROBLEM

PROJECT: _____ RAN BY: _____ DATE: _____

ENTER (R/S)

H (feet)	STMT	DI (inch)	D (inch)	Soil H	HS	a
20.0	12.0	2.5	10.0	0.03 %	2.0	1.76

$f_c = 3000$ #/sq in
 $f_s = 60,000$ #/sq in

PRINT

STEM DESIGN

Nº	SECTION HEIGHT (FT)	MOMENT (KI)	SECTION THICK. (IN)	As (req'd) $\frac{M}{\phi d}$	As (min.) $= .0025bt$
1	2.0	0.16	13.00	0.0087	0.39 #
2	4.0	0.80	14.00	0.0395	0.420
3	6.0	2.16	15.00	0.0982	0.45
4	8.0	4.48	16.00	0.1886	0.48
5	10.0	8.00	17.0	0.3135	0.51
6	12.0	12.96	18.0	0.475	0.54
7	14.0	19.60	19.0	0.674	0.57
8	16.0	28.16	20.0	0.914	0.60
9	18.0	38.88	21.0	1.194	0.63
10	20.0	52.00	22.0	1.515	0.66

ENTER (R/S) PRINT

K	M (allow)	V	SHT (feet)
204	77.57 KI	0.0308 %	1.00

ENTER (R/S)

TRIAL FOOTING

ENTER (R/S) PRINT ENTER (R/S)

TP (feet)	HP (feet)	FTG WIDTH	FTG	P	Z
3.0	5.0	9.833	1.833	0	0

PRINT FOOTING REQUIRED (FIX U8)

TP (feet)	HP (feet)	FTG WIDTH	FTG
4.0	6.0	11.833	1.833

PRINT

M_0	BASE SHEAR
66.34 KI	8.46 K

PRINT

M_R (kft.)	FACTOR SAFE. OVERTURNING	W	$\bar{X}$ LOCAT. OF RESULTANT	f_p (max.)	f_p (min.)
153.09 KI	2.3078	21.153 K	4.10 FT	3.433 K/ft	0.1421 K/ft

PRINT

M (heel)	A_s (heel)	M (allow)	V (heel)	USE
31.97 KI	0.982 #	69.78 KI	0.0405 %	#9 @ 12

PRINT

M (toe)	A_s (toe)	V (toe)	USE	COEFF OF FRICTION	Passive Press.	KEY DEPTH
21.499 KI	0.660 #	0.0451 %	#8 @ 12	0.45	3.503 K	2.0 FT

WEEP HOLES 4" ϕ @ 10'-0" O/C

FDN PIPE 4" ϕ PREFORATED

KEY SIZE 1'-10" x 2'-0"

FOOTING TEMP STEEL TOP BOT #7 @ 12 #7 @ 12

Shear Not Problem !!

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PUSH LABEL A, INCREMENTS HEEL AND TOE

PUSH LABEL B, HOLDS TOE PROJECTION (TP)

PUSH LABEL C, HOLDS HEEL PROJECTION (HP)

PUSH LABEL D IF SURCHARGE IS TO BE INCLUDED IN RESISTING MOMENT.

1

PROJECT: _____ RAN BY: _____ DATE: _____

ENTER (R/S)

H (feet)	STMT	DI (inch)	D (inch)	Soil H	HS	a
20	12	2.5	10	0.03 %	2.0	1.76

PRINT

STEM DESIGN

N ^o	SECTION HEIGHT (FT)	MOMENT (kl)	SECTION THICK. (IN)	As (req'd) $\frac{M}{\phi a d}$	As (min) $= 0.025 b d$
1	2.0	0.16	13.0	0.0087	0.39
2	4.0	0.80	14.0	0.0395	0.420
3	6.0	2.16	15.0	0.0982	0.450
4	8.0	4.48	16.0	0.1886	0.480
5	10.0	8.00	17.0	0.3135	0.510
6	12.0	12.96	18.0	0.475	0.540
7	14.0	19.60	19.0	0.6749	0.570
8	16.0	28.16	20.0	0.9143	0.60
9	18.0	38.88	21.0	1.1941	0.63
10	20.0	52.00	22.0	1.5152	0.66

ENTER (R/S) PRINT

K	M (allow)	V	SHT (feet)
204	77.57 ^{kl}	0.0308 %	1.00

ENTER (R/S)

TRIAL FOOTING

ENTER (R/S) PRINT

TP (feet)	HP (feet)	FTG WIDTH	FTG	P	Z
4.0	5.5	11.3333	1.833	0 ^{kl}	0

PRINT FOOTING REQUIRED (FIX 08)

TP (feet)	HP (feet)	FTG WIDTH	FTG
4.0	5.75	11.583	1.833

PRINT

M ₀	BASE SHEAR
66.34 ^{kl}	8.460 ^{kl}

PRINT

MR (kft.)	FACTOR SAFE. OVERTURNING	W	X LOCAT. OF RESULTANT	f _p (max.)	f _p (min.)
146.43 ^{kl}	2.2074	20.584 ^{kl}	3.891 FT	3.526 ^{kl}	0.0275 ^{kl}

PRINT

M (heel)	A _s (heel)	M (allow)	V (heel)	USE
30.89 ^{kl}	0.9488 ^{kl}	69.78 ^{kl}	0.0409 %	#9@12

PRINT

M (toe)	A _s (toe)	V (toe)	USE	COEF. OF FRICTION	Passive Press.	KEY DEPTH
21.99 ^{kl}	0.675 ^{kl}	0.0459 %	#8@12	0.45	3.5037 ^{kl}	2.0 FT

WEEP HOLES

4" @ 10'-0" %

FDN PIPE

4" PREFORATED

KEY SIZE

1'-10" x 2'-0"

FOOTING TEMP STEEL TOP BOT

#7@12 #7@12

Shear Not Problem!!

Program Listing

STEP	KEY ENTRY	KEY CODE	COMMENTS	STEP	KEY ENTRY	KEY CODE	COMMENTS
	01*LBL "RETWALL"		Retaining Wall	52	PRX		
	02 CF 00			53	10		
	03*LBL A		Increments Heel	54	/		
	04 CF 01		and Toe	55	STO 21		%
	05 CF 02			56	"SOIL H"		
	06 GTO F			57	PROMPT		
	07*LBL B		Hold Toe and	58	PRX	←	Enter Soil H
	08 "HOLD TOE"		Increments	59	STO 04		
	09 AVIEW		Heel	60	"HS"		
	10 SF 01			61	PROMPT	←	Enter HS
	11 CF 02			62	PRX		
	12 GTO F			63	STO 05		
	13*LBL C		Hold Heel and	64	"a"		
	14 "HOLD HEEL"		Increments Toe	65	PROMPT	←	Enter a
	15 AVIEW			66	PRX		= fsj/12000
	16 SF 02			67	STO 06		
	17 CF 01			68	1		
	18 GTO F			69	STO 09		start Count
	19*LBL D		Adds Surcharge	70*LBL 11			
	20 "ADD SURCHARGE"		to resisting	71	ADV		
	21 AVIEW		Moment if	72	RCL 09		
	22 SF 00		Flag ϕ Set	73	PRX	→	Print Count
	23 STOP			74	RCL 00	→	Print H _N
	24*LBL F			75	PRX	→	Distance from top
	25 SF 11			76	X↑2		of wall
	26 SF 12			77	RCL 04		
	27 "CALPRO="			78	*		
	28 PRA			79	2		
	29 "CALCULATOR PRO"			80	/		
	30 "I-GRAMS"			81	RCL 05		
	31 PRA			82	RCL 00		
	32 CF 12			83	3		
	33 ADV			84	/		
	34 "H"			85	+		
	35 PROMPT	←	Enter H	86	*		
	36 PRX			87	PRX	→	Print M
	37 10			88	STO 22		
	38 /		H ₁₀	89	RCL 03		
	39 STO 20			90	RCL 21		
	40 STO 00			91	+		
	41 "STMT"			92	PRX	→	Print STMT+D _N
	42 PROMPT	←	Enter Stmt	93	STO 03		
	43 PRX			94	RCL 02		
	44 STO 01			95	-		
	45 STO 03			96	STO 00		
	46 "D1"			97	/		
	47 PROMPT	←	Enter D1	98	RCL 06		
	48 PRX			99	/		
	49 STO 02			100	PRX	→	Print As = $\frac{M}{ad}$
	50 "D"			101	RCL 03		
	51 PROMPT	←	Enter D	102	.03		
				103	*		
				104	PRX	→	As(min) = .0025bd
				105	RCL 20		= .03d
				106	ST+ 00		
				107	1		

REGISTERS

00 H _N	01 STMT TP	02 D1 HP	03 STMT+D _N	04 Soil H	05 HS	06 a	07 MR	08 d W	09 Count FTG
10 M ₀	11	12 P	13 Z	14	15 FTA WIDTH	16	17	18	19
20 H ₁₀	21 D ₁₀	22 M K	23 V _{base} Shear	24 M	25 SHT	26	27	28	29
30	31	32	33	34	35	36	37	38	39
40	41	42	43	44	45	46	47	48	49
50	51	52	53	54	55	56	57	58	59

STEP	KEY ENTRY	KEY CODE	COMMENTS	STEP	KEY ENTRY	KEY CODE	COMMENTS
108	ST+ 09			164	12		
109	RCL 09			165	/		
110	11			166	+		
111	-			167	"FTG WIDTH"		
112	X<0?		Bottom of Wall?	168	AVIEW		
113	GTO 11		H ₁₀	169	PRX		Print FTg WIDTH
114	ADV			170	STO 15		
115	"K"			171	"FTG"		
116	PROMPT		Enter K	172	PROMPT		Enter FTg
117	PRX			173	PRX		
118	STO 22			174	STO 09		
119	RCL 08			175	RCL 00		
120	X↑2			176	+		FTg + H
121	*			177	STO 20		
122	3			178	X↑2		
123	10↑X			179	RCL 04		
124	/			180	*		Soil H x H ² / ₂
125	"M ALLOW"			181	2		
126	AVIEW			182	/		
127	PRX		$M(allow) = \frac{d^2 K(12)}{12000}$	183	RCL 05		
128	RCL 00			184	RCL 20		$M_o = Soil H \times \frac{H^2}{2} \times$
129	RCL 20			185	3		$(H_s + \frac{H}{3})$
130	-			186	/		
131	STO 00		H ₁₀ = H ₁₁ - H _N	187	+		
132	2		Height of wall	188	*		M _o
133	/			189	STO 10		
134	RCL 05			190	RCL 20		
135	+			191	2		
136	RCL 00			192	/		
137	*			193	RCL-05		
138	RCL 04		$U = \frac{Soil H \times H_N (H_s + \frac{H_N}{3})}{12 \times d_N}$	194	+		
139	*			195	RCL 04		
140	12			196	*		
141	/		$\frac{STMT+D}{12}$	197	RCL 20		
142	RCL 08			198	*		
143	/			199	STO 23		
144	"V"			200	12		
145	AVIEW		Print U	201	ST/ 03		
146	PRX		Enter SHT	202	"P"		Enter "P"
147	"SHT"			203	PROMPT		
148	PROMPT			204	PRX		
149	PRX			205	STO 12		
150	STO 25			206	"Z"		Enter Z
151	ADV			207	PROMPT		
152	"TRIAL FOOTING"		Trial Footing	208	PRX		
153	AVIEW			209	STO 13		
154	"TP"						
155	PROMPT		Enter TP	210	LBL G		
156	PRX			211	RCL 05		
157	STO 01			212	RCL 00		
158	"H P"			213	FS? 00		
159	PROMPT		Enter HP	214	+		
160	PRX			215	.1		
161	STO 02			216	*		
162	+			217	RCL 02		
163	RCL 03			218	*		

LABELS					FLAGS		SET STATUS							
A	B	C	D	E	0	1	FLAGS		TRIG		DISP			
✓	✓	✓	✓		✓	✓	ON	OFF	DEG	■	FIX	■		
F	✓	G	✓	J	2	3	1	■	GRAD	□	SCI	□		
a	b	c	d	e	4	5	2	■	RAD	□	ENG	□		
01	✓	02	✓	03	6	7	3	■			n	4		
06	07	08	09	10	8	9	10	11	12	13	14	15		
11	✓	12	✓	13	16	17	18	19	20	21	22	23		
16	17	18	19	20	24	25	26	27	28	29	30	31		
21	22	23	24	25										

Add Surcharge
in Resisting
Moment if
Flag & set

STEP	KEY ENTRY	KEY CODE	COMMENTS	STEP	KEY ENTRY	KEY CODE	COMMENTS
219	STO 08		$Wgt = 0.1 \times HP(H+H_s)$	289	-		
220	LASTX			290	LASTX		
221	2			291	ABS		$\frac{M_{net} - Ftg \text{ width}}{W}$
222	/		$M_N = Wgt \times \left(\frac{HP}{2} + TP \right)$	292	RCL 15		Check Kern
223	RCL 01		$\frac{+ STMT + DN}{12}$	293	6		Increment Ftg
224	+			294	/		if outside of
225	RCL 03			295	-		Kern
226	+			296	X>0?		
227	*			297	GTO H		
228	STO 07		M_N	298	RCL 10		
229	.15			299	"M-0"		
230	RCL 00			300	AVIEW		
231	*		$Wgt \text{ Stem} = 0.15H \times$	301	PRX	→	Moveturning
232	RCL 03		$\left(\frac{STMT + DN}{12} \right)$	302	RCL 23		
233	*			303	"BASE SHEAR"		
234	ST+ 08		Moment Arm	304	AVIEW		
235	LASTX		$= Wgt \times \left(\frac{HP}{2} + TP \right)$	305	PRX	→	Print Base
236	2		$\frac{+ STMT + D}{12 \times 2}$	306	ADV		Shear
237	/			307	"FOOTING REQD"		Footing Req'd
238	RCL 01			308	AVIEW		
239	+			309	RCL 01		
240	*			310	"TP"		
241	ST+ 07		M_{wall}	311	AVIEW		
242	.15			312	PRX	→	Print TP
243	RCL 09			313	RCL 02		
244	*			314	"HP"		
245	RCL 15			315	AVIEW		
246	*			316	PRX	→	Print HP
247	ST+ 08		$Wgt \text{ Ftg} = .15 \times Ftg$	317	RCL 15		
248	LASTX		$\times (FTG \text{ WIDTH})$	318	"FTG WIDTH"		
249	*			319	AVIEW		
250	2		$M(FTG)$	320	PRX	→	Print FTG
251	/		$M_{Footing}$	321	RCL 09		WIDTH
252	ST+ 07			322	"FTG"		
253	.1			323	AVIEW		
254	RCL 25			324	PRX	→	Print FTG
255	*			325	RCL 07		
256	RCL 01			326	ADV		
257	*			327	"M-R"		
258	ST+ 08		W_{TOE}	328	AVIEW		
259	RCL 01			329	PRX	→	Print MR
260	*			330	RCL 10		
261	2			331	/		
262	/			332	"F.S."		
263	ST+ 07		M_{TOE}	333	AVIEW		
264	RCL 12		P	334	PRX	→	Print Factor
265	ST+ 08			335	RCL 08		Safety
266	RCL 13			336	"W"		overturning
267	*			337	AVIEW		
268	ST+ 07		$MP = P \times Z$	338	PRX	→	Print W
269	RCL 07			339	RCL 07		Weight
270	RCL 08			340	RCL 10		
271	/			341	-		
272	STO 20			342	RCL 08		
273	RCL 15			343	/		
274	2			344	"X"		
275	/			345	AVIEW		
276	-		$e = \frac{MR}{W} - \frac{Ftg \text{ width}}{2}$	346	PRX	→	Print "X"
277	RCL 08			347	RCL 24		Location of
278	*		$M_{net} = We - M_o$	348	RCL 15		Resultant
279	RCL 10			349	/		
280	-			350	6		$\bar{X} = \frac{M - M_o}{W}$
281	CHS			351	*		
282	STO 24			352	STO 20		
283	RCL 08		$\bar{X} = \frac{M_{net}}{W} + \frac{Ftg \text{ width}}{2}$	353	RCL 08		
284	/			354	+		$F_p = \frac{P}{A} \pm \frac{M}{S}$
285	RCL 15			355	RCL 15		$= \left(P \pm \frac{GM}{Ftg \text{ width}} \right) \left(\frac{1}{Ftg \text{ width}} \right)$
286	2			356	/		
287	/			357	"F P MAX"		$F_p(\max)$
288	X<Y			358	AVIEW		

STEP	KEY ENTRY	KEY CODE	COMMENTS	STEP	KEY ENTRY	KEY CODE	COMMENTS
359	PRX	→	Print $F_p(\max)$	429	/		
360	STO 04			430	RCL 22		
361	RCL 08			431	*		
362	RCL 20			432	"M ALLOW"		
363	-			433	AVIEW		
364	RCL 15			434	PRX	→	$M_{allow} = \frac{Kd^2}{1000}$
365	/			435	RCL 05		
366	"F P MIN"			436	12		
367	AVIEW			437	/		
368	PRX	→	Print $F_p(\min)$	438	RCL 20		$v = \frac{V}{12d}$
369	ADV			439	/		
370	STO 03			440	"V HEEL"		
371	-			441	AVIEW		
372	RCL 15			442	PRX	→	Print v_{Heel}
373	/			443	ADV		
374	RCL 02			444	RCL 25		
375	X+2			445	.1		
376	*			446	*		
377	2			447	RCL 09		
378	/			448	.15		
379	CHS	→	Triangular Load	449	*		
380	RCL 05			450	+		
381	STO 24			451	CHS		
382	RDN			452	RCL 04		
383	STO 05			453	+		
384	RCL 02			454	RCL 01		
385	3			455	*		
386	/			456	STO 05	→	stores Load
387	*			457	RCL 01		
388	STO 07			458	*		
389	RCL 24			459	2		
390	RCL 00			460	/		
391	+			461	STO 07	→	$M_{Toe} = \frac{W_{Toe}(TP)}{2}$
392	.1			462	RCL 04		
393	*			463	RCL 03		
394	RCL 03			464	-		
395	-			465	RCL 15		
396	.15			466	/		
397	RCL 09			467	RCL 01		
398	*			468	X+2		
399	+			469	*		
400	RCL 02			470	2		
401	*			471	/		
402	ST+ 05			472	ST- 05		
403	RCL 02			473	RCL 01		
404	*			474	3		
405	2			475	/		
406	/			476	*		
407	ST+ 07			477	ST- 07		
408	RCL 07			478	RCL 07		
409	"HEEL"			479	"TOE"		
410	AVIEW			480	AVIEW		
411	"M"			481	"M TOE"		
412	AVIEW			482	AVIEW		
413	PRX	→	Print M_{Heel}	483	PRX	→	M_{Toe}
414	RCL 09			484	RCL 06		
415	12			485	/		
416	*			486	RCL 20		
417	3.5			487	/		
418	-			488	"A S TOE"		
419	STO 20			489	AVIEW		
420	/			490	PRX	→	$A_s = \frac{M}{ad}$
421	RCL 06			491	RCL 05		
422	/			492	12		
423	"A S HEEL"			493	/		
424	AVIEW			494	RCL 20		
425	PRX	→	Print $A_s = \frac{M}{ad}$	495	/		
426	RCL 20			496	"V TOE"		
427	X+2			497	AVIEW		
428	1000			498	PRX	→	$v_{Toe} = \frac{V_{Toe}}{bd}$

STEP	KEY ENTRY	KEY CODE	COMMENTS	STEP	KEY ENTRY	KEY CODE	COMMENTS
	499 Ø		Start Shear key				
	500 STO 06		Design				
	501 RCL 23						
	502 1.5		Safety factor				
	503 *		Sliding = 1.5				
	504 RCL 08						
	505 .45		$V * 1.5 - 0.45 W = 0$				
	506 "KEY"						
	507 AVIEW						
	508 "COEF FRICTION"						
	509 AVIEW						
	510 PRX →		Print Coef of				
	511 *		friction = 0.45				
	512 -						
	513 STO 20						
	514 *LBL 13		Increments key				
	515 RCL 20		depth in 3"				
	516 RCL 25		Increments				
	517 RCL 09						
	518 +						
	519 RCL 06						
	520 +						
	521 X12						
	522 .15		$P = \frac{0.3 H^2}{2} = .15 H^2$				
	523 *						
	524 X/Y?		Stop when Passive				
	525 GTO J		Pressure exceeds				
	526 .25		Horiz Load * 1.5				
	527 ST+ 06						
	528 GTO 13						
	529 *LBL J						
	530 "PASSIVE PRESSUR"						
	531 AVIEW						
	532 PRX →		Passive Pressure				
	533 RCL 06						
	534 "KEY DEPTH"						
	535 AVIEW						
	536 PRX →		Print Key depth				
	537 ADV						
	538 ADV						
	539 ADV						
	540 ADV						
	541 GTO A →		Returns to A				
	542 *LBL H						
	543 .25		Hold Toe?				
	544 FS? 01		Hold Heel?				
	545 GTO 01						
	546 FS? 02						
	547 GTO 02						
	548 ST+ 13		Increments Heel,				
	549 ST+ 01		Toe, z, Ftg Width,				
	550 ST+ 02		Ftg Width by 0.25 feet				
	551 ST+ 15						
	552 ST+ 15						
	553 GTO G						
	554 *LBL 01						
	555 ST+ 02		Increments Heel				
	556 ST+ 15		+ Ftg Width by				
	557 GTO G		3"				
	558 *LBL 02						
	559 ST+ 15		Increments Toe,				
	560 ST+ 13		Ftg Width, and				
	561 ST+ 01		z by 3"				
	562 GTO 07						
	563 .END.						

FOOTING - AXIAL LOAD "FTGAL"
PROGRAM DESCRIPTION

2-1

Program calculates pier reinforcement, footing size, soil pressure, footing reinforcement, moments, shears, number of bars, area per bar, and punching shear for both square and rectangular footings. Uses Working Stress Design (Alternate Design Method) or Ultimate Strength Design (Load Factor Design) when divided by the appropriate load factors.

If you should desire a printout of the prompting statements along with the numerical results, set your printer to the normal mode.

The main program may be addressed directly by the execute key (XEQ) and the program name "FTGAL". Flag eleven has been set causing the program to initialize immediately upon entry of program cards.

Working Stress Design (Alternate Design Method) or Ultimate Strength Design (Load Factor Design) when divided by the appropriate load factor can be used to determine the reinforcement, shear, and moment capacities. Remember that both "K" and "a" must be divided by the appropriate Load Factor prior to entry into the calculator. Sheet iii-1 has been included in the front of this manual for ready reference of WSD and USD Values for "a", "K", " v_o " and " v_c " for 3000 PSI, 4000 PSI, and 5000 PSI concrete.

For positive loads, footing size is increased until the external axial load, footing, earth, and pier loads yield a soil pressure less than the allowable soil bearing value.

All negative loads are multiplied by 1.5 to assure an adequate safety factor against uplift. The footing size is increased until a positive pressure exists on bottom of footing based on a soil weight of 100 LBS/FT³ and concrete weight of 150 LBS/FT³.

The footing thickness is not increased internally, so the actual moments and shears must be compared with the allowables to insure that footing is capable of carrying the load with an adequate factor of safety. Normally, the punching shears will govern first, the beam shears second and moments last.

The maximum punching shear is computed for a concrete strength of 3000 PSI so that a comparison can be made with the external axial load. When " v_o " is less than the axial load "P", you must either increase footing thickness and/or manually calculate the additional bearing capacity under footing within the punching shear area at "d/2". You must also increase footing thickness when the moment capacity or the beam shear exceeds the allowables.

Program will check several loads for the same footing without inputting the parameters each time. However, if the footing size is altered after you input the second load, simply re-enter the earlier load to obtain the new footing requirements.

Three inch clearance is used for all bars assuming a maximum bar size of 1"; d for main bars is 3.5" and 4.5" for transverse bars.

Where negative values occur for uplift loads, place this amount of steel in top of footing.

A_s (min) pier = .005 x gross pier area
 footing = .002 x gross footing area

L = Footing length (feet)

W = Footing width (feet)

T = Footing thickness (feet)

Earth Thickness = Thickness of earth on top of footing (feet)

B = Width of pier (inches)

C = Length of pier (inches)

A_s = Area of reinforcement (in²)

$a = f_s j / 12000$ for Working Stress Design

$a_u / 1.7 = \phi f_y (1 - .59w) / (12,000 \times 1.7)$ for Ultimate Strength Design

$K = \frac{1}{2} f_c k j = M / F = M / b d^2$ for Working Stress Design

$= M_u / (F \times 1.7) = \phi f'_c w (1 - .59w) / 1.7 = K_u / 1.7$ for Ultimate Strength Design

P = External load (kips)

F_p = Actual soil bearing value under footing (KSF)

v = Shear (KSI)

M = Moment (Kip-feet)

Soil Brg Value = Allowable Soil Bearing Value

JOHN DOE & ASSOCIATES
CONSULTING STRUCTURAL ENGINEERS
111 YOUR STREET
ANYWHERE, U.S.A. 00000

JOB SAMPLE PROBLEM
SHEET NO. _____ OF _____
CALCULATED BY _____ DATE _____
CHECKED BY _____ DATE _____
SCALE _____ COMM. NO. _____

FOOTING LOCATION <u>FOOTING K-27</u> (PUSH LABEL A) PUSH LABEL <u>B</u> FOR HOLDING WIDTH OF FOOTING.	FOOTING AXIAL LOAD 2
---	--------------------------------

ENTER (R/S)

L	W	T	Earth Thickness	Soil Brq Value
<u>6.0</u> FT	<u>4.0</u> FT	<u>1.17</u> FT	<u>2</u> FT	<u>3</u> $\frac{K}{ft^2}$

ENTER (R/S) PRINT ENTER (R/S)

B	C	As(Pier)	a	K
<u>18</u> IN	<u>16</u> IN	<u>1.44</u> in^2	<u>1.76</u>	<u>204</u>

.005 BC

ENTER (R/S) PRINT

P1	L	W	T	FP
<u>80</u> K	<u>6.75</u> FT	<u>4.75</u> FT	<u>1.17</u> FT	<u>2.876</u> $\frac{K}{ft^2}$

BOTTOM MAIN FTG REINF

As(TOTAL)	Nº BARS	As/Bar	$u@d$	As(min)	M(actual)	M=FK
<u>2.349</u> in^2	<u>5</u>	<u>0.4698</u> in^2	<u>0.0362</u> $\frac{K}{ft}$	<u>1.6006</u> in^2	<u>9.17</u> $\frac{Kl}{ft}$	<u>22.66</u> $\frac{Kl}{ft}$

$M_{act} \leq FK$

BOTTOM TRANSVERSE FTG REINF

As(TOTAL)	Nº BARS	As/Bar	$u@d$	As(min)	M(actual)	M=FK
<u>1.327</u> in^2	<u>7</u>	<u>0.1897</u> in^2	<u>0.0181</u> $\frac{K}{ft}$	<u>2.2745</u> in^2	<u>3.30</u> $\frac{Kl}{ft}$	<u>18.56</u> $\frac{Kl}{ft}$

$M_{act} \leq FK$

ENTER (R/S) FOR SECOND LOAD

P2	L	W	T	FP
<u>-4</u> K	<u>6.75</u> FT	<u>4.75</u> FT	<u>1.170</u> FT	<u>0.1940</u> $\frac{K}{ft^2}$

ENTER (R/S) PRINT
LIFT

BOTTOM MAIN FTG REINF

As(TOTAL)	Nº BARS	As/Bar	$u@d$	As(min)	M(actual)	M=FK
<u>-0.1699</u> in^2	<u>5</u>	<u>-0.0340</u> in^2	<u>-0.0026</u> $\frac{K}{ft}$	<u>1.6006</u> in^2	<u>-0.663</u> $\frac{Kl}{ft}$	<u>22.66</u> $\frac{Kl}{ft}$

$M_{act} \leq FK$

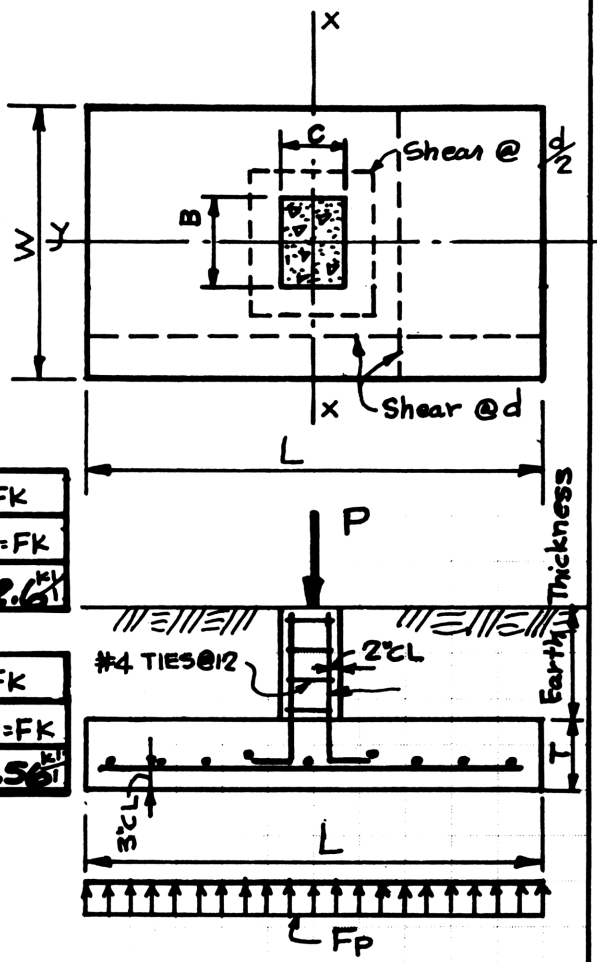
BOTTOM TRANSVERSE FTG REINF

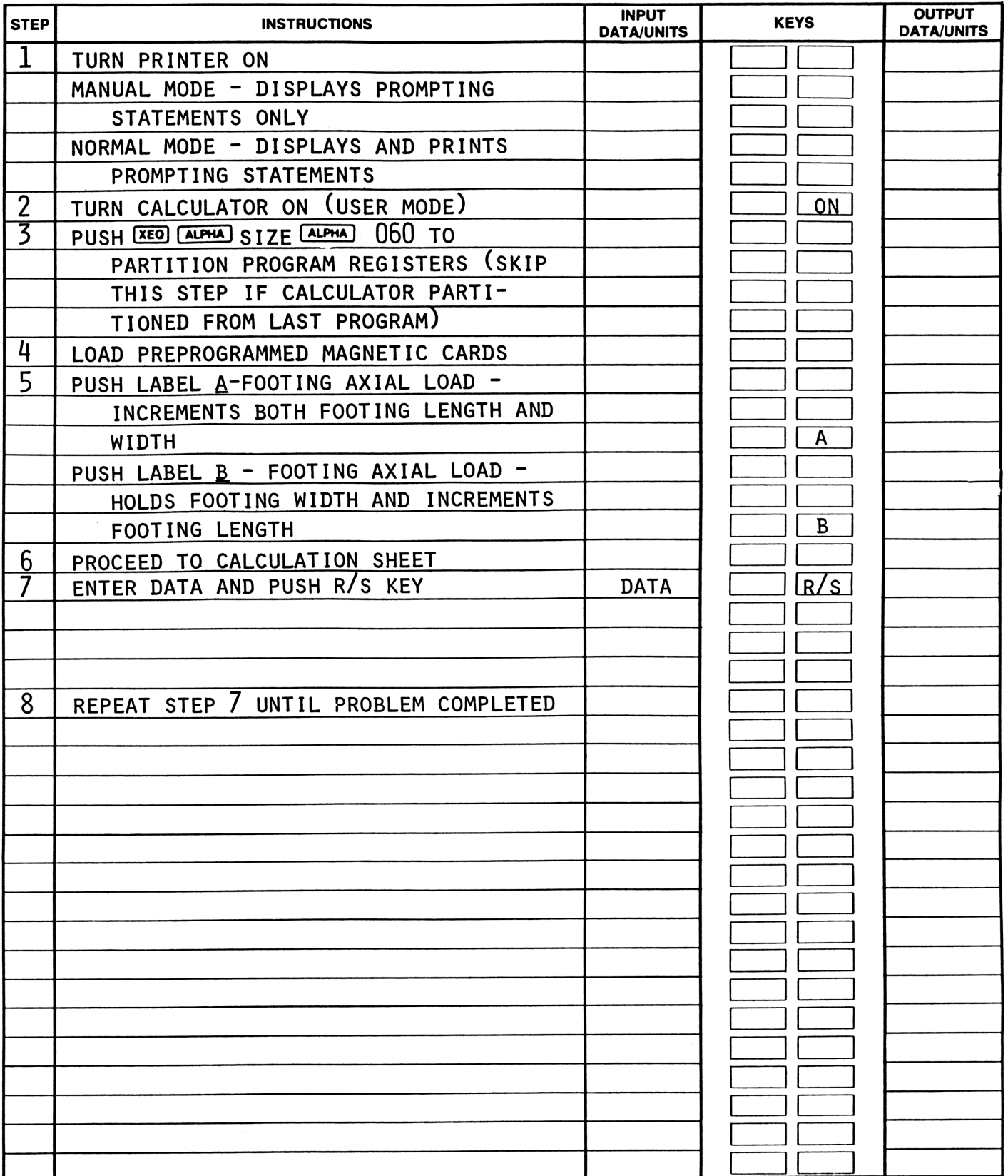
As(TOTAL)	Nº BARS	As/Bar	$u@d$	As(min)	M(actual)	M=FK	$V \geq P_2$
<u>-0.096</u> in^2	<u>7</u>	<u>-0.0137</u> in^2	<u>-0.0013</u> $\frac{K}{ft}$	<u>2.2745</u> in^2	<u>-0.238</u> $\frac{Kl}{ft}$	<u>18.566</u> $\frac{Kl}{ft}$	<u>111.4043</u> K

$V_{allow} @ \frac{1}{2} = 0.11 b_o d$

IF FOOTING PARAMETERS DO NOT CHANGE, YOU CAN ENTER AS MANY LOADS AS DESIRED WITH FOOTING SIZES AND REINFORCEMENT COMPUTED FOR EACH LOAD.

USE Ftg 6'-9" x 4'-9" w/ 6-#6 L.W.B ; 5-#4 L.W.T
7-#4 S.W.B ; 7-#4 S.W.T





STEP	KEY ENTRY	KEY CODE	COMMENTS	STEP	KEY ENTRY	KEY CODE	COMMENTS												
	01*LBL "FTGAL"		Footing-axial Load "FTGAL"		54 *		$WL \left[\frac{0.15 \times T}{144} + \frac{BC}{144} \right] + \frac{BC}{144} (0.05)$ Weight of Ftg, = WL [0.15 x T + 0.1 x (Earth Thickness)] + BC (Earth Thickness) (0.05) Soil & Pier soil pressure ≤ than allowable?												
	02*LBL A				55 RCL 02														
	03 SF 00			56 .15															
				57 *															
	04*LBL 11			58 RCL 03															
	05 SF 11			59 .1															
	06 SF 12			60 *															
	07 "=CALPRO="			61 +															
	08 PRA			62 *															
	09 "CALCULATOR PRO"			63 STO 09															
	10 "GRAMS"			64 RCL 05															
	11 PRA			65 RCL 06															
	12 CF 12			66 *															
	13 ADV			67 144															
	14 0			68 /															
	15 STO 25		count	69 RCL 03															
	16 "L"			70 *															
	17 XEQ 00	←	Enter L	71 .05															
	18 "W"			72 *															
	19 XEQ 00	←	Enter W	73 ST+ 09															
	20 "T"			74 RCL 20															
	21 XEQ 00	←	Enter T	75 RCL 09															
	22 "EARTH THICKNESS"			76 +															
	23 XEQ 00	←	Earth Thickness	77 RCL 00															
	24 "SOIL BRG VALUE"			78 /															
	25 XEQ 00	←	Soil Brq Value	79 RCL 01															
	26 ADV			80 /															
	27 "B"			81 STO 21	-soil pressure														
	28 XEQ 00	←	Enter B	82 XEQ 00	uplift on Footing?														
	29 "C"			83 GTO 03															
	30 XEQ 00	←	Enter C	84 RCL 04															
	31 *			85 X=Y?															
	32 .005			86 GTO 03															
	33 *			87 RCL 00															
	34 "A S"			88 "L"															
	35 AVIEW			89 AVIEW															
	36 PRX	→	Print $A_s = .005 BC$ Pier	90 PRX	→	Print L													
	37 "a"			91 RCL 01															
	38 XEQ 00	←	Enter a	92 "W"															
	39 "K"			93 AVIEW															
	40 XEQ 00	←	Enter K	94 PRX	→	Print W													
	41 ADV			95 RCL 02															
				96 "T"															
	42*LBL C			97 AVIEW															
	43 "P"			98 PRX	→	Print T													
	44 PROMPT	←	Enter P	99 RCL 03															
	45 PRX			100 .1															
	46 XEQ 00			101 *															
	47 XEQ 02			102 RCL 02															
	48 STO 20			103 .15															
	49 "FOOTING SIZE"			104 *															
	50 AVIEW			105 +															
				106 RCL 21															
	51*LBL 12			107 "F P"															
	52 RCL 00			108 AVIEW															
	53 RCL 01			109 PRX	→	Print F_p soil Brq Value													
REGISTERS																			
00	L	01	W	02	T	03	Earth Thickness	04	Soil Brq Value	05	B	06	C	07	a	08	K	09	Ftg Weight
10		11		12		13		14		15		16		17		18		19	
20	P	21	W' Soil Pressure	22	W' Soil Pressure	23	d	24		25	Count Moment Area	26		27		28		29	
30		31		32	causing Bending	33		34		35		36		37		38		39	
40		41		42		43		44		45		46		47		48		49	
50		51		52		53		54		55		56		57		58		59	

uplift on Footing?

$$\text{Weight of Ftg,} = WL \left[0.15 \times T + 0.1 \times (\text{Earth Thickness}) \right] + \frac{BC (\text{Earth Thickness})}{144} (0.05)$$

soil pressure $\leq$ than allowable?

STEP	KEY ENTRY	KEY CODE	COMMENTS	STEP	KEY ENTRY	KEY CODE	COMMENTS	
110	ADV		Soil pressure causing Bending	166	.11		Print Vallow(d_z) = 0.11 b _{od} Go To E	
111	X<Y			167	*			
112	-			168	"V ALLOW"			
113	STO 22			169	AVIEW			
114	RCL 00			170	PRX			
115	RCL 06			$d = T \times 12 - 3.5"$	171	ADV		Prints and Stores Data
116	XEQ 04				172	ADV		
117	RCL 01				173	ADV		
118	*				174	GTO C		
119	RCL 02				175	LBL 00		
120	12		176		PROMPT		Provide Safety Factor of 1.5 for uplift	
121	*		177		PRX			
122	3.5		178		STO IND 25			
123	-		179		71SZ			
124	STO 23		180		RTN			
125	/		MAIN Print $A_s = \frac{M}{ad}$	181	LBL 02		Add 3" to Length Add 3" to width if Flag ϕ set	
126	RCL 07			182	1.5			
127	/			183	*			
128	"MAIN"			184	RTN			
129	AVIEW			185	LBL 03			
130	PRX			$d_T = d_m - 1"$	186	.25		M = $W \times \frac{w_{Bending}}{2}$ $\times \left[\frac{L - G/2}{2} \right]^2$
131	RCL 01				187	ST+ 00		
132	XEQ 05				188	FS? 00		
133	RCL 01				189	ST+ 01		
134	XEQ 06				190	GTO 12		
135	RCL 22		Transverse Print $A_s = \frac{M}{ad}$		191	LBL 04		Print No Bars Print A_s /bar
136	RCL 01				192	12		
137	RCL 05				193	/		
138	XEQ 04				194	-		
139	RCL 00				195	2		
140	*			196	/		Print No Bars Print A_s /bar	
141	RCL 23			197	STO 25			
142	1			198	X+2			
143	-			199	*			
144	STO 23			200	2			
145	/		201	/				
146	RCL 07		202	STO 24				
147	/		203	RTN				
148	"TRANSVERSE"		Print No Bars Print A_s /bar	204	LBL 05			Print No Bars Print A_s /bar
149	AVIEW			205	1			
150	PRX			206	+			
151	RCL 00			207	INT			
152	XEQ 05			208	PRX			
153	RCL 00			209	/			
154	XEQ 06			210	PRX			
155	RCL 05			211	RCL 25			
156	RCL 06			212	RCL 23			
157	+			213	12			
158	RCL 23		214	/				
159	+		215	-				
160	RCL 23		216	RCL 22				
161	+							
162	2							
163	*							
164	RCL 23							
165	*							

LABELS					FLAGS		SET STATUS								
A	B	C	D	E	0	1	FLAGS			TRIG			DISP		
☒	☒	☒					ON OFF			DEG	GRAD	RAD	FIX	SCI	ENG
F	G	H	I	J	2	3	0	☐	☐	☐	☐	☐	☐	☐	☐
a	b	c	d	e	4	5	1	☐	☐	☐	☐	☐	☐	☐	☐
00 ☒	01 ☒	02 ☒	03 ☒	04 ☒	6	7	2	☐	☐	☐	☐	☐	☐	☐	☐
06 ☒	07 ☒	08	09	10	8	9	3	☐	☐	☐	☐	☐	☐	☐	☐
11 ☒	12 ☒	13	14	15	16	17									
16	17	18	19	20	24	25									
21	22	23	24	25											

STEP	KEY ENTRY	KEY CODE	COMMENTS	STEP	KEY ENTRY	KEY CODE	COMMENTS
217	*		Beam Shear @ d				
218	12		$v = w' \times (L' - d/12)$				
219	/		12" x d				
220	RCL 23						
221	/						
222	PRX →		Print Shear @				
223	RTN		d from Face				
224	LBL 06						
225	RCL 02						
226	*						
227	.288						
228	*						
229	PRX →		$A_s(\min) = .002 WT$				
230	RCL 24						
231	"MOMENT"						
232	AVIEW						
233	PRX →		Print Moment				
234	RCL 08		K'/FT				
235	RCL 23						
236	X12		$M = \frac{bd^2}{12000} \times K$				
237	*						
238	E3						
239	/						
240	PRX →		Print M = FK				
241	ADV						
242	RTN						
243	LBL 8		Clear flag &				
244	CF 00		to Hold Footing				
245	GTO 11		Width				
246	.END.						

ECCENTRICALLY LOADED FOOTING "ECCFTGC"

COMBINED LOADS

PROGRAM DESCRIPTION

Program calculates minimum and maximum pressures for three loading combination ($D + L$, $D + W$, $D + L + W$, or others) including axial loads, horizontal shear at top of pier, and moment; pier reinforcement; footing reinforcement each way top and bottom; number of bars; area of bars; and shears. This program will automatically increment footing size until soil bearing value or uplift requirements are satisfied. You can increment footing width and length or the length only. Uses Working Stress or Ultimate Strength Design divided by the appropriate load factor per ACI 318-77.

If you should desire a printout of the prompting statements along with the numerical results, set your printer to the normal mode.

The main program may be addressed directly by the execute key (XEQ) and the program name "ECCFTGC". Flag eleven has been set causing the program to initialize immediately upon entry of program cards.

Footing length and width are incremented in 3" increments until the actual soil bearing value is less than the allowable soil bearing value. When uplift exists the footing is also incremented until a positive pressure exists under the entire footing. A factor of safety is not applied internally to uplift loads and is usually not warranted if a moment and/or horizontal shear exists. If only an uplift load exists, apply a safety factor of 1.5 or use the "Footing-Axial Load" program.

Each loading condition is complete in itself, which means you can input any loading combination that you desire including $D + L$, $D + W$, $D + L + W$ and others.

A three inch clearance is used for all bars assuming a maximum bar size of 1"; d for main bars is 3.5" and 4.5" for transverse bars.

This program uses Working Stress Design (Alternate Design Method) or Ultimate Strength Design (Load Factor Design) when divided by the load factor of 1.7 to determine the reinforcement and moment capacities. Remember that both " K " and " a " must be divided by the appropriate load factor prior to entry into the calculator. Sheet iii-1 has been included in the front of this manual for ready reference of WSD and USD values for " a ", " K ", " v_o " and " v_c " for 3000 PSI, 4000 PSI, and 5000 PSI concrete.

The footing or pier thickness is not increased internally, so the actual moments and shears must be compared with the allowables to insure that footing and pier can resist the loads with an adequate factor of safety. The program will not design for compressive reinforcement so the actual moments should never exceed the allowable moments.

Top reinforcement is designed to totally resist the earth and concrete load when lifted vertically, without regard to pressure on bottom which would serve to reduce the reinforcement required.

This program treats the pier within the ground as an earth load. If the pier is excessively large, add 0.05 K/FT^3 for volume below ground line and $.15 \text{ K/FT}^3$ for volume above the ground line.

IMPORTANT! The maximum pressures must always occur on the toe as the moments and shears are determined from the toe pressures. The program will automatically check each of the three conditions for maximum moment and maximum shear. However, the maximum moments and shears do not have to occur in the same loading condition but must occur on the toe side of the footing. The results obtained by inputting the moment and shear on top of the pier in the opposite direction from that shown will give you moments and shears based on the minimum pressure similar to the heel of a retaining wall.

Nomenclature:

Shear at $d/2 = 0.11 \times b_o \times d$

$M_o = M + H (T + \text{Earth thickness} + Z)$

$$F_p = \frac{P}{L \times W} \pm \frac{6M_o}{W \times L^2}$$

L = Footing length (feet)

W = Footing width (feet)

T = Footing thickness (feet)

Earth thickness = thickness of earth on footing (feet)

Z = Height of pier above the ground or floor line (feet)

P = Axial load (kips)

M = Overturning moment on pier (kip feet)

H = Horizontal load on top of pier (kips)

F_p = Actual soil pressure under footing (kips/SF)

B = Pier width (inches)

C = Pier length (inches)

A_s = Area of reinforcement; $M/a \times d$ or $M/(a_u \times 1.7 \times d)$ (in^2)

v = Beam shear at face of pier or punching shear at $d/2$ (kips/in^2)

$a = f_s j / 12000$ for Working Stress Design

$= a_u / 1.7 = \phi f_y (1 - .59w) / (12,000 \times 1.7)$ for Ultimate Strength Design

$K = \frac{1}{2} f_c k j = M/F = M/bd^2$ for Working Stress Design

$= M_u / (F \times 1.7) = \phi f'_c w (1 - .59w) / 1.7 = K_u / 1.7$ for Ultimate Strength Design

$F = bd^2 / 12000$

f'_c = Compressive Concrete strength (PSI)

f_y = Yield Strength of reinforcement (PSI)

A_s (min) - Footing = $.002 \times \text{Gross Footing Area}$
 Pier = $.005 \times \text{Gross Pier Area}$

JOHN DOE & ASSOCIATES
CONSULTING STRUCTURAL ENGINEERS
111 YOUR STREET
ANYWHERE, U.S.A. 00000

JOB SAMPLE PROBLEM

SHEET NO. _____ OF _____

CALCULATED BY _____ DATE _____

CHECKED BY _____ DATE _____

SCALE _____ COMM. NO. _____

FOOTING LOCATIONS: **FOOTING B-7**
(PUSH LABEL A)

ECCENTRICALLY LOADED FOOTING
(COMBINED LOADS)

3

PUSH LABEL B FOR HOLDING WIDTH OF FOOTING.
ENTER (R/S)

DEAD + LIVE

L	W	T	Earth Thick	Z	Soil Brg Value	P _{DL+LL}	M _{DL+LL}	H _{DL+LL}
4.0 FT	6.0 FT	1.5 FT	2.0 FT	3 FT	4 $\frac{K}{ft^2}$	150 K	20 KI	6 K

DEAD + WIND

P _{DL+WL}	M _{DL+WL}	H _{DL+WL}
75 K	5 KI	2 K

DEAD + LIVE + WIND

P _{DL+LL+WL}	M _{DL+LL+WL}	H _{DL+LL+WL}	$\frac{a_u}{1.7}$ (USD)	$\frac{K_u}{1.7}$ (USD)
P _{DL+LL+WL}	M _{DL+LL+WL}	H _{DL+LL+WL}	a (WSD)	K (WSD)
125.0 K	7 KI	3 K	1.76	204

FOOTING REQUIRED (PRINT)

L	W	T
6.75 FT	8.75 FT	1.50 FT

DEAD + LIVE (PRINT)

P _{DL+LL}	M _{DL+LL}	H _{DL+LL}	F _{p(max)}	F _{p(min)}
150.0 K	20.0 KI	6.0 K	3.852 $\frac{K}{ft^2}$	2.076 $\frac{K}{ft^2}$

DEAD + WIND

P _{DL+WL}	M _{DL+WL}	H _{DL+WL}	F _{p(max)}	F _{p(min)}
75.0 K	5.0 KI	2.0 K	1.9657 $\frac{K}{ft^2}$	1.4239 $\frac{K}{ft^2}$

DEAD + LIVE + WIND

P _{DL+LL+WL}	M _{DL+LL+WL}	H _{DL+LL+WL}	F _{p(max)}	F _{p(min)}
125.0 K	7.0 KI	3.0 K	2.9402 $\frac{K}{ft^2}$	2.1426 $\frac{K}{ft^2}$

PIER; ENTER (R/S) PRINT

B	C	A _{s(min)}	M _(max)	A _s or A' _s	REINF	Ties	Sketch
16 IN	18 IN	1.44 $\frac{in^2}{ft}$	50.0 KI	1.8939 $\frac{in^2}{ft}$	- #	#4@12	

.005 BC

BOTT MAIN FTG REINF

BOTT TRANSVERSE FTG REINF

BOTTOM MAIN

A _{s(TOTAL)}	No. Bars	A _{s/Bar}	U _{face Pier}	A _{s(TOTAL)}	No. Bars	A _{s/Bar}	U _{face Pier}	A _{s(min)}	M _(max)	M=FK
3.7771 $\frac{in^2}{ft}$	9	0.4197 $\frac{in^2}{ft}$	0.0465 $\frac{in^2}{ft}$	6.6954 $\frac{in^2}{ft}$	7	0.9565 $\frac{in^2}{ft}$	0.0785 $\frac{in^2}{ft}$	3.780 $\frac{in^2}{ft}$	11.016 KI	42.891 KI

↑ HIGH, INCREASE FTG T

BOTTOM TRANSVERSE

TOP MAIN REINF

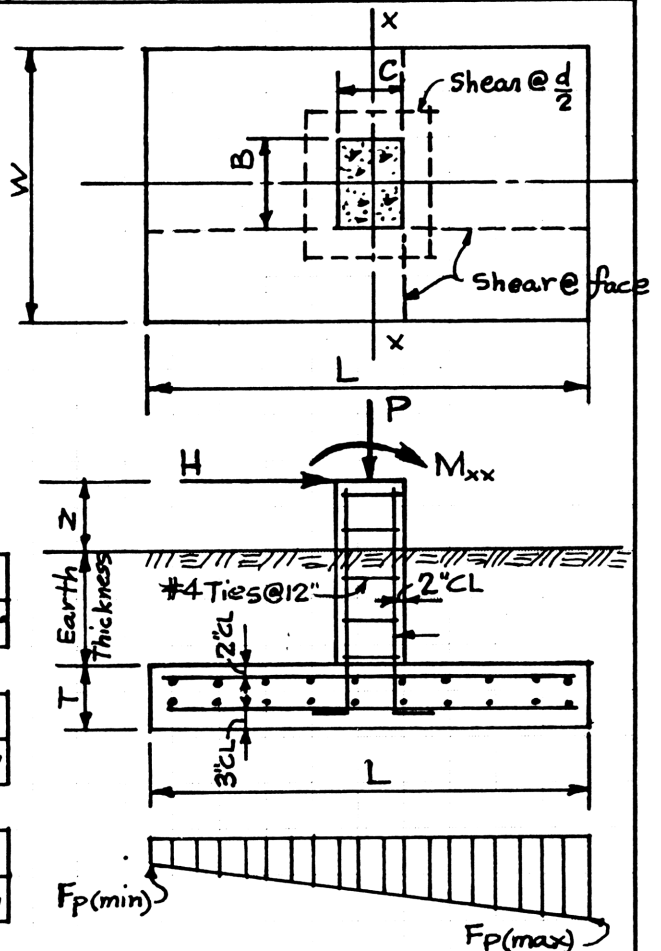
TOP TRANSVERSE REINF

Val_{bure d}

A _{s(min)}	M _(max)	M=FK	A _s	No. Bars	A _{s/Bar}	A _s	No. Bars	A _{s/Bar}	Val _{bure d}	f _c
2.916 $\frac{in^2}{ft}$	23.56 KI	37.17 KI	0.502 $\frac{in^2}{ft}$	9	0.0558 $\frac{in^2}{ft}$	0.8302 $\frac{in^2}{ft}$	7	0.1186 $\frac{in^2}{ft}$	190.96 K	3,000 $\frac{psi}{ft^2}$

INCREASE FTG THICKNESS !!!

f_y = 60,000 $\frac{psi}{ft^2}$



[illegible]

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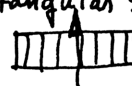
STEP	KEY ENTRY	COMMENTS	STEP	KEY ENTRY	KEY CODE	COMMENTS
	01*LBL "ECCFTGC"	Eccentrically		55 PRX		
	02*LBL A	Loaded Footing		56 STO 12		
	03 SF 00	"ECCFTGC"		57 "DEAD + WIND"		Dead + Wind Load
	04 GTO F			58 PRA		
				59 "P"		
	05*LBL B	Clear Flag & to		60 PROMPT	←	P _{DL} +WL
	06 CF 00	Hold width		61 PRX		
				62 STO 13		
	07*LBL F			63 "M"		
	08 SF 11			64 PROMPT	←	M _{DL} +WL
	09 SF 12			65 PRX		
	10 "=CALPRO="			66 STO 14		
	11 PRA			67 "H"		
	12 "CALCULATOR PRO"			68 PROMPT	←	H _{DL} +WL
	13 "+GRAMS"			69 PRX		
	14 PRA			70 STO 15		
	15 CF 12			71 "DEAD+LIVE+WIND"		Dead + Live + Wind
	16 ADV			72 PRA		
	17 "L"			73 "P"		
	18 PROMPT	← Enter L		74 PROMPT	←	P _{DL} +LL+WL
	19 PRX			75 PRX		
	20 STO 00			76 STO 16		
	21 "W"			77 "M"		
	22 PROMPT	← Enter W		78 PROMPT	←	M _{DL} +LL+WL
	23 PRX			79 PRX		
	24 STO 01			80 STO 17		
	25 "T"			81 "H"		
	26 PROMPT	← Enter T		82 PROMPT	←	H _{DL} +LL+WL
	27 PRX			83 PRX		
	28 STO 02			84 STO 18		
	29 "EARTH THICK"			85 "a"		
	30 PROMPT	← Earth thickness		86 PROMPT	←	Enter a
	31 PRX			87 PRX		
	32 STO 03			88 STO 26		
	33 "Z"			89 "K"		
	34 PROMPT	← Enter Z		90 PROMPT	←	Enter K
	35 PRX			91 PRX		
	36 +			92 STO 27		
	37 +			93 ADV		
	38 STO 24			94 "FOOTING REQD"		Footing Required
	39 "SOIL BRG"			95 AVIEW		
	40 PROMPT	← Enter Soil Brg Value				
	41 PRX			96*LBL 15		
	42 STO 21			97 10		
	43 "DEAD + LIVE"	Dead + Live Load		98 STO 25		
	44 PRA			99 XEQ 11		Calculates P _{DL} +LL
	45 "P"			100 XEQ 12		Footing Requirements
	46 PROMPT	← P _{DL} +LL		101 ST+ 04		
	47 PRX			102 ST- 05		
	48 STO 10			103 RDN		
	49 "H"			104 ST+ 04		
	50 PROMPT	← M _{DL} +LL		105 ST+ 05		
	51 PRX			106 RCL 04		
	52 STO 11			107 RCL 21		
	53 "H"			108 X>Y?		Checks for Toe
	54 PROMPT	← H _{DL} +LL		109 GTO 16		Pressure > soil Brg Value

Program Listing

HP-41C

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STEP	KEY ENTRY	KEY CODE	COMMENTS	STEP	KEY ENTRY	KEY CODE	COMMENTS
	215 STO 06			281 *			
	216 STO 07			282 RCL 11			
	217 STO 08			283 +			
	218 STO 09			284 STO 23			M_{DL+LL}
	219 RCL 00			285 RCL 15			
	220 RCL 01			286 RCL 19			
	221 *			287 *			
	222 STO 22		$Area = L \times W$	288 RCL 14			
	223 RCL 00		$S = Area \times \frac{L}{6}$	289 +			
	224 *			290 RCL 23			
	225 6			291 $X \leftarrow Y?$			
	226 /			292 $X \leftarrow Y$			
	227 STO 23			293 STO 23			M_{DL+WL}
	228 RTN			294 RCL 18			
				295 RCL 19			
				296 *			
	229*LBL 12			297 RCL 17			
	230 RCL IND 25		Finds P/A	298 +			
	231 RCL 22			299 RCL 23			$M_{DL+LL+WL}$
	232 /			300 $X \leftarrow Y?$			
	233 7ISZ		Finds M/S	301 $X \leftarrow Y$			
	234 RCL IND 25			302 "M"			
	235 7ISZ			303 PRA			
	236 RCL IND 25		Changes H to	304 PRX →			Prints M_{max}
	237 RCL 24		Moment	305 RCL 21			
	238 *		$M + H(T + \frac{Earth}{THK} + Z)$	306 3			
	239 +		S	307 -			$d = c - 3"$
	240 RCL 23			308 /			
	241 /			309 XEQ 29 →			Print A_s
	242 7ISZ			310 ADV			
	243 RTN			311 0			Zero in
				312 STO 11			Storage Registers
	244*LBL 13		Adds 3" to Flg	313 STO 12			
	245 .25		Length	314 STO 14			
	246 ST+ 00		Adds 3" to Flg	315 STO 15			
	247 FS? 00		width if Flag & set	316 RCL 25			
	248 ST+ 01			317 RCL 21			
	249 RTN			318 "BOTTOM"			Bottom
				319 PRA			
	250*LBL 18			320 XEQ 28			$c/12"$
	251 RCL IND 25		Prints	321 STO 21			
	252 PRX		Values	322 -			
	253 7ISZ			323 2			
	254 RTN			324 /			
				325 STO 18			$(L-c)/2$
	255*LBL G			326 RCL 22			
	256 "PIER"		Pier	327 RCL 20			
	257 PRA			328 XEQ 28			
	258 "B"			329 STO 20			
	259 PROMPT ↔		Enter B	330 -			
	260 PRX			331 2			
	261 STO 20			332 /			
	262 "C"			333 STO 13			$(W-B)/2$
	263 PROMPT ↔		Enter C	334 .1			
	264 PRX			335 RCL 03			
	265 STO 21			336 *			
	266 *			337 .15			
	267 .005			338 RCL 02			Calculate Unit
	268 *			339 *			weight of soil
	269 "A S"			340 +			& Footing
	270 PRA			341 STO 03			
	271 PRX →		$A_s = .005 BC$	342 XEQ 21			$DL+LL$
	272 RCL 00		min	343 RCL 06			Moments & Shears
	273 STO 25			344 STO 04			
	274 RCL 01			345 RCL 07			
	275 STO 22			346 STO 05			
	276 RCL 24		Moment Arm	347 XEQ 21			$DL+WL$
	277 RCL 02			348 RCL 08			Moments & Shears
	278 -			349 STO 04			
	279 STO 19			350 RCL 09			
	280 RCL 12						

STEP	KEY ENTRY	KEY CODE	COMMENTS	STEP	KEY ENTRY	KEY CODE	COMMENTS
351	STO 05		DL+LL+WL Moment & Shears Recalls Mmax $d = T \times 12 - 3.5$	420	2		M Stores Mmax during Each Call
352	XEQ 21			421	/		
353	11			422	RCL 23		
354	STO 25			423	-		
355	RCL IND 25			424	71SZ		
356	RCL 02			425	RCL IND 25		Stores Vmax
357	12			426	X<=Y?		
358	*			427	X<>Y		
359	3.5			428	STO IND 25		
360	-			429	71SZ		
361	STO 04		Print $A_s = \frac{M}{ad} \times W$ Bars & Area/Bar Prints shear = $\frac{V}{12d}$ M-Transverse $d_{Trans} = d - 1"$ Print $A_s = \frac{M}{ad} \times L$ Bars & Area/Bar Prints shear = $\frac{V}{12d}$	430	RCL 22		Calculates Transverse shear Max @ Toe
362	/			431	RCL IND 25		
363	RCL 01			432	X<=Y?		
364	*			433	X<>Y		
365	XEQ 29			434	STO IND 25		
366	RCL 01			435	71SZ		Store Max shear
367	XEQ 22			436	RCL 04		
368	RCL 04			437	RCL 03		
369	/			438	-		
370	PRX			439	RCL IND 25		
371	71SZ		Calculates Mmax then stores & Returns	440	*		Calculates Mmax then stores & Returns
372	71SZ			441	71SZ		
373	RCL IND 25			442	71SZ		
374	RCL 00			443	RCL IND 25		
375	*			444	X<=Y?		
376	RCL 04			445	X<>Y		Calculates No of Reinf Bars
377	1			446	STO IND 25		
378	-			447	LASTX		
379	STO 06			448	*		
380	/			449	2		
381	XEQ 29		Calculates Triangular Shear  $V_A = \frac{f_{pmax} - f_{pmin}}{2L} \left(\frac{L-C}{2} \right)^2$	450	/		Bars As/Bar
382	RCL 00			451	7DSZ		
383	XEQ 22			452	STO IND 25		
384	RCL 06			453	RTN		
385	/			454	LBL 22		Divides by 12
386	PRX			455	1		
387	GTO H			456	+		
388	LBL 21			457	INT		
389	10			458	"BARS"		
390	STO 25		Rectangular Shear  $M_A = \frac{VL}{3}$ $[f_{pmax} - wgt] \times \frac{L-C}{2} = V$ $M_{Rectangular} = \frac{VL}{2}$	459	PRA		Print $A_s = \frac{M}{ad}$
391	RCL 04			460	PRX		
392	RCL 05			461	/		
393	-			462	"A S/BAR"		
394	RCL 00			463	PRA		
395	/			464	PRX		As = 0.002 x 144 x W x T
396	RCL IND 25			465	71SZ		
397	*			466	RCL IND 25		
398	RCL IND 25			467	LBL 28		
399	*			468	12		
400	2		Prints shear = $\frac{V}{12d}$	469	/		Prints shear = $\frac{V}{12d}$
401	/			470	RTN		
402	STO 22			471	LBL 29		
403	RCL IND 25			472	"A S"		
404	*			473	PRA		
405	3			474	RCL 26		As = 0.002 x 144 x W x T
406	/			475	/		
407	STO 23			476	PRX		
408	RCL 04			477	RTN		
409	RCL 03			478	LBL H		
410	-		As = 0.002 x 144 x W x T	479	ADV		As = 0.002 x 144 x W x T
411	RCL IND 25			480	RCL 02		
412	*			481	RCL 01		
413	RCL 22			482	*		
414	-			483	.288		
415	STO 22			484	*		As = 0.002 x 144 x W x T
416	LASTX			485	PRX		
417	+						
418	RCL IND 25						
419	*						

STEP	KEY ENTRY	KEY CODE	COMMENTS	STEP	KEY ENTRY	KEY CODE	COMMENTS
486	11			555	*		
487	STO 25			556	RCL 06		
488	RCL IND 25			557	+		
489	PRX →		Print $M_{max} @ Toe$	558	+		
490	RCL 04			559	RCL 04		
491	X↑2			560	RCL 06		
492	RCL 27			561	+		
493	*			562	*		
494	1000			563	.11		
495	/			564	*		
496	PRX →		$M_{allow} = FK$	565	"V ALLOW"		
497	RCL 02		$= \frac{bd^2}{12000} \times K$	566	PRA		
498	RCL 00			567	PRX →		Print V_{allow}
499	*			568	ADV		on footing
500	.288			569	ADV		
501	*			570	GTO A		
502	ADV			571	.END.		
503	PRX →		$AS = 0.002 \times 144 \times L \times T$				
504	14						
505	STO 25						
506	RCL IND 25						
507	PRX →		$M_{max} Transverse$				
508	RCL 06						
509	X↑2						
510	RCL 27						
511	*						
512	1000						
513	/						
514	PRX →		$M_{allow} = \frac{bd^2}{12000} \times K$				
515	10						
516	STO 25						
517	RCL 03						
518	RCL IND 25		Ftg wgt & Soil wgt				
519	X↑2		$= w$				
520	*						
521	2						
522	/		$M = wL^2/2$				
523	RCL 04						
524	/						
525	RCL 01						
526	*						
527	"TOP"		Top				
528	PRA						
529	XEQ 29 →		$As = \frac{M}{ad} \times W$				
530	RCL 01						
531	XEQ 22 →		No & Area of Bars				
532	13						
533	STO 25						
534	RCL 03						
535	RCL IND 25						
536	X↑2						
537	*						
538	2						
539	/						
540	RCL 06						
541	/						
542	RCL 00						
543	*						
544	XEQ 29 →		Print $As = \frac{M}{ad}$				
545	RCL 00						
546	XEQ 22 →		No & Area of bar				
547	ADV						
548	RCL 21						
549	12						
550	*						
551	RCL 04						
552	+						
553	RCL 20						
554	12						

$$b_o = [(B+c) \times 12 + d_{main} + d_{transverse}] \times 2$$

$$V_{allow} = b_o d \times v_c$$

$$v_c = 2\sqrt{f_c} = 0.11 \frac{K}{in}$$

ECCENTRICALLY LOADED FOOTING "ECCFTGI"

INDIVIDUAL LOADS

PROGRAM DESCRIPTION

Program combines loads ($D + L$, $D + W$, and $D + L + W$) then calculates minimum and maximum pressures for three loading combinations including axial loads, horizontal shear at top of pier, and moment; pier reinforcement, footing reinforcement each way top and bottom; number of bars; areas of bars; shears. This program will automatically increment footing size until soil bearing value or uplift requirements are satisfied. You can increment footing width and length or the length only. Uses Working Stress or Ultimate Strength Design divided by the appropriate load factor per ACI 318-77.

If you should desire a printout of the prompting statements along with the numerical results, set your printer to the normal mode.

The main program may be addressed directly by the execute key (XEQ) and the program name "ECCFTGI". Flag eleven has been set causing the program to initialize immediately upon entry of program cards.

Footing length and width are incremented in 3" increments until the actual soil bearing value is less than the allowable soil bearing value. When uplift exists the footing is also incremented until a positive pressure exists under the entire footing. A factor of safety is not applied internally to uplift loads and is usually not warranted if a moment and/or horizontal shear exists. If only an uplift load exists, apply a safety factor of 1.5 or use the "Footing-Axial Load" program.

This program will combine the loads into three combinations $D + L$, $D + W$, $D + L + W$ and then analyzes the footing.

Three inch clearance is used for all bars assuming a maximum bar size of 1"; d for main bars is 3.5" and 4.5" for transverse bars.

This program used Working Stress Design (Alternate Design Method) or Ultimate Strength Design (Load Factor Design) when divided by the load factor of 1.7 to determine the reinforcement and moment capacities. Remember that both " K " and " a " must be divided by the appropriate load factor prior to entry into the calculator. Sheet iii-1 has been included in the front of this manual for ready reference of WSD and USD values for " a ", " K ", " v_o ", and " v_c " for 3000 PSI, 4000 PSI, 4000 PSI, and 5000 PSI concrete.

The footing or pier thickness is not increased internally, so the actual moments and shears must be compared with the allowables to insure that footing and pier can resist the loads with an adequate factor of safety. The program will not design for compressive reinforcement so the actual moments should never exceed the allowable moments.

Top reinforcement is designed to totally resist the earth and concrete load when lifted vertically, without regard to pressure on bottom which would serve to reduce the reinforcement required.

This program treats the pier within the ground as an earth load. If the pier is excessively large, add 0.05 K/FT^3 for volume above the ground line.

IMPORTANT! The maximum pressures must always occur on the toe as the moments and shears are determined from the toe pressures. The program will automatically check each of the three conditions for maximum moment and maximum shear. However, the maximum moments and shears do not have to occur in the same loading condition but must occur on the toe side of the footing. The results obtained by inputting the moment and shear on top of the pier in the opposite direction from that shown will give you moments and shears based on the minimum pressure similar to the heel of a retaining wall.

Nomenclature:

Shear at $d/2 = 0.11 \times b_o \times d$

$M_o = M + H (T + \text{Earth thickness} + Z)$

$$F_p = \frac{P}{L \times W} \pm \frac{6M_o}{W \times L^2}$$

L = Footing length (feet)

W = Footing width (feet)

T = Footing thickness (feet)

Earth thickness = thickness of earth on footing (feet)

Z = Height of pier above the ground or floor line (feet)

P = Axial load (kips)

M = Overturning moment on pier (kip feet)

H = Horizontal load on top of pier (kips)

F_p = Actual soil pressure under footing (kips/SF)

B = Pier width (inches)

C = Pier length (inches)

A_s = Area of reinforcement; $M/a \times d$ or $M/(a_u \times 1.7 \times d)$ (in^2)

v = Beam shear at face of pier or punching shear at $d/2$ (kips/in^2)

$a = f_s j / 12000$ for Working Stress Design

$= a_u / 1.7 = \phi f_y (1 - .59w) / 12,000 \times 1.7$ for Ultimate Strength Design

$K = \frac{1}{2} f_c k j = M/F = M/bd^2$ for Working Stress Design

$= M_u / (F \times 1.7) = \phi f'_c w(1 - .59w) / 1.7 = K_u / 1.7$ for Ultimate Strength Design

$F = bd^2 / 12000$

f'_c = Compressive Concrete strength (PSI)

f_y = Yield Strength of reinforcement (PSI)

A_s (min) - Footing = $.002 \times \text{Gross Footing Area}$
 Pier = $.005 \times \text{Gross Pier Area}$

JOHN DOE AND ASSOCIATES
CONSULTING STRUCTURAL ENGINEERS
111 YOUR STREET
ANYWHERE, U.S.A.

JOB: SAMPLE PROBLEM

4-3

SHEET NO. _____ OF _____

CALCULATED BY _____ DATE _____

CHECKED BY _____ DATE _____

SCALE _____ COMM. NO. _____

FOOTING LOCATIONS:
(PUSH LABEL A)

FOOTING Z2

ECCENTRICALLY LOADED FOOTING
(INDIVIDUAL LOADS)

4

PUSH LABEL B FOR HOLDING WIDTH OF FOOTING.
ENTER (R/S)

DEAD LOAD

L	W	T	Earth Thick	Z	Soil Brg Value	P _{DL}	M _{DL}	H _{DL}
4.0 FT	6.0 FT	1.67 FT	2.0 FT	1.0 FT	3.0 $\frac{K}{ft^2}$	100 K	10 KI	2.0 K

LIVE LOAD

P _{LL}	M _{LL}	H _{LL}
50 K	5.0 KI	1.0 K

WIND LOAD

P _{WL}	M _{WL}	H _{WL}	$\frac{a_u}{1.7}$ (USD)	$\frac{K_u}{1.7}$ (USD)
-25 K	-10 KI	-0.5 K	a (WSD)	K (WSD)
			1.44	226

FOOTING REQUIRED (PRINT)

L	W	T
7.5 FT	9.5 FT	1.67 FT

DEAD + LIVE (PRINT)

P _{DL+LL}	M _{DL+LL}	H _{DL+LL}	F _{p(max)}	F _{p(min)}
150 K	15 KI	3 K	2.8815 $\frac{K}{ft^2}$	2.23 $\frac{K}{ft^2}$

DEAD + WIND

P _{DL+WL}	M _{DL+WL}	H _{DL+WL}	F _{p(max)}	F _{p(min)}
75 K	0.0 KI	1.5 K	1.5818 $\frac{K}{ft^2}$	1.4245 $\frac{K}{ft^2}$

DEAD + LIVE + WIND

P _{DL+LL+WL}	M _{DL+LL+WL}	H _{DL+LL+WL}	F _{p(max)}	F _{p(min)}
125 K	5.0 KI	2.5 K	2.3921 $\frac{K}{ft^2}$	2.0177 $\frac{K}{ft^2}$

PIER; ENTER (R/S) PRINT

B	C	A _{s(min)}	M _(max)	A _s or A _{s'}	REINF	Ties	Sketch
16 IN	18 IN	1.44 $\frac{in^2}{ft}$	24 KI	1.1111 $\frac{in^2}{ft}$	- #	#4@12	

.005 BC

BOTT MAIN FTG REINF

BOTT TRANSVERSE FTG REINF

BOTTOM MAIN

A _{s(TOTAL)}	No. Bars	A _{s/Bar}	U _{face Pier}	A _{s(TOTAL)}	No. Bars	A _{s/Bar}	U _{face Pier}	A _{s(min)}	M _(max)	M=FK
4.2075 $\frac{in^2}{ft}$	10	0.4207 $\frac{in^2}{ft}$	0.0348 $\frac{in^2}{ft}$	6.7925 $\frac{in^2}{ft}$	8	0.8491 $\frac{in^2}{ft}$	0.053 $\frac{in^2}{ft}$	4.5691 $\frac{in^2}{ft}$	10.548 KI	61.827 KI

BOTTOM TRANSVERSE

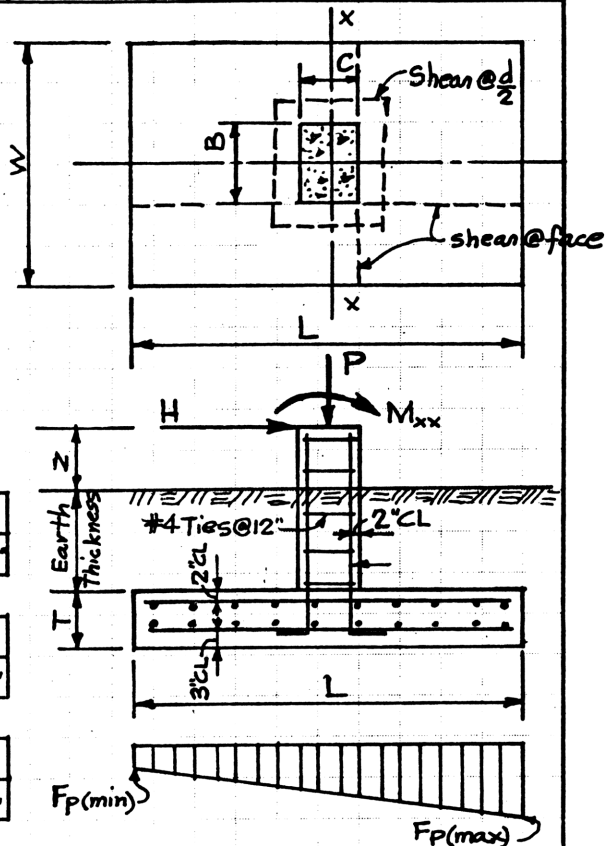
TOP MAIN REINF

TOP TRANSVERSE REINF

V_{allowed}

A _{s(min)}	M _(max)	M=FK	A _s	No. Bars	A _{s/Bar}	A _s	No. Bars	A _{s/Bar}	=all b.d	f _{c'}
3.6072 $\frac{in^2}{ft}$	20.26 KI	54.57 KI	0.8086 $\frac{in^2}{ft}$	10	0.0809 $\frac{in^2}{ft}$	1.2598 $\frac{in^2}{ft}$	8	0.1575 $\frac{in^2}{ft}$	233.18 K	3,000 $\frac{psi}{ft^2}$

f_y = 40,000 $\frac{psi}{ft^2}$



NO. CARDS

[illegible]

[illegible]

PAGE 4-7

STEP	KEY ENTRY	KEY CODE	COMMENTS	STEP	KEY ENTRY	KEY CODE	COMMENTS
	227 STO 06				293 *		
	228 STO 07				294 RCL 11		
	229 STO 08				295 +		
	230 STO 09				296 STO 23		M_{DL+LL}
	231 RCL 00				297 RCL 15		
	232 RCL 01				298 RCL 19		
	233 *				299 *		
	234 STO 22		$Area = L \times W$		300 RCL 14		
	235 RCL 00		$S = Area \times \frac{L}{6}$		301 +		
	236 *				302 RCL 23		
	237 6				303 X<=Y?		
	238 /				304 X<>Y		
	239 STO 23				305 STO 23		M_{DL+WL}
	240 RTN				306 RCL 18		
					307 RCL 19		
					308 *		
	241+LBL 12				309 RCL 17		
	242 RCL IND 25		Finds P_A		310 +		
	243 RCL 22				311 RCL 23		$M_{DL+LL+WL}$
	244 /				312 X<=Y?		
	245 7ISZ				313 X<>Y		
	246 RCL IND 25		Finds M/S		314 "M"		
	247 7ISZ				315 PRA		
	248 RCL IND 25				316 PRX		Prints M_{max}
	249 RCL 24		Changes H to		317 RCL 21		
	250 *		Moment		318 3		
	251 +		$M + H(T + \frac{Earth}{THK} + Z)$		319 -		$d = C - 3"$
	252 RCL 23				320 /		
	253 /				321 XEQ 29		Print A_s
	254 7ISZ				322 ADV		
	255 RTN				323 0		Zero in
					324 STO 11		Storage Registers
	256+LBL 13				325 STO 12		
	257 .25		Adds 3" to Flg		326 STO 14		
	258 ST+ 00		Length		327 STO 15		
	259 FS? 00		Adds 3" to Flg		328 RCL 25		
	260 ST+ 01		width if Flag & set		329 RCL 21		
	261 RTN				330 "BOTTOM"		Bottom
					331 PRA		
	262+LBL 18				332 XEQ 28		$C/12\%$
	263 RCL IND 25		Prints		333 STO 21		
	264 PRX		Values		334 -		
	265 7ISZ				335 2		
	266 RTN				336 /		
					337 STO 10		$(L-C)/2$
	267+LBL G				338 RCL 22		
	268 "PIER"		Pier		339 RCL 20		
	269 PRA				340 XEQ 28		
	270 "B"				341 STO 20		
	271 PROMPT		Enter B		342 -		
	272 PRX				343 2		
	273 STO 20				344 /		
	274 "C"				345 STO 13		$(W-B)/2$
	275 PROMPT		Enter C		346 .1		
	276 PRX				347 RCL 03		
	277 STO 21				348 *		
	278 *				349 .15		
	279 .005				350 RCL 02		
	280 *				351 *		
	281 "A S"				352 +		
	282 PRA				353 STO 03		
	283 PRX		$A_s = .005 BC$		354 XEQ 21		
	284 RCL 00		min		355 RCL 06		
	285 STO 25				356 STO 04		
	286 RCL 01				357 RCL 07		
	287 STO 22				358 STO 05		
	288 RCL 24				359 XEQ 21		
	289 RCL 02				360 RCL 08		
	290 -				361 STO 04		
	291 STO 19				362 RCL 09		
	292 RCL 12		Moment Arm				

STEP	KEY ENTRY	KEY CODE	COMMENTS	STEP	KEY ENTRY	KEY CODE	COMMENTS
363	STO 05		DL+LL+WL	432	2		
364	XEQ 21		Moment & Shears	433	/		
365	11			434	RCL 23		M
366	STO 25		Recalls M _{max}	435	-		Stores M _{max}
367	RCL IND 25			436	7ISZ		during Each Call
368	RCL 02			437	RCL IND 25		
369	12		$d = T \times 12 - 3.5$	438	X<=Y?		
370	*			439	X<>Y		
371	3.5			440	STO IND 25		
372	-			441	7ISZ		
373	STO 04			442	RCL 22		Stores V _{max}
374	/			443	RCL IND 25		
375	RCL 01			444	X<=Y?		
376	*			445	X<>Y		
377	XEQ 29	→	Print $A_s = \frac{M}{ad} \times W$	446	STO IND 25		
378	RCL 01			447	7ISZ		
379	XEQ 22	→	Bars & Area/Bar	448	RCL 04		Calculates
380	RCL 04			449	RCL 03		Transverse shear
381	/			450	-		Max @ Toe
382	PRX	→	Prints shear = $\frac{V}{12d}$	451	RCL IND 25		
383	7ISZ			452	*		
384	7ISZ			453	7ISZ		
385	RCL IND 25		M _{Transverse}	454	7ISZ		
386	RCL 00			455	RCL IND 25		Store Max shear
387	*			456	X<=Y?		
388	RCL 04			457	X<>Y		
389	1			458	STO IND 25		
390	-			459	LASTX		Calculates M _{max}
391	STO 06		$d_{trans} = d - 1"$	460	*		then stores &
392	/			461	2		Returns
393	XEQ 29	→	Print $A_s = \frac{M}{ad} \times L$	462	/		
394	RCL 00			463	7DSZ		
395	XEQ 22	→	Bars & Area/Bar	464	STO IND 25		
396	RCL 06			465	RTN		
397	/						
398	PRX	→	Prints shear = $\frac{V}{12d}$				
399	GTO H						
400	LBL 21			466	LBL 22		Calculates No
401	10			467	1		of Reinf Bars
402	STO 25		Calculates	468	+		
403	RCL 04		Triangular Shear	469	INT		
404	RCL 05			470	"BARS"		
405	-			471	PRA		Bars
406	RCL 00			472	PRX	→	
407	/			473	/		
408	RCL IND 25			474	"A S/BAR"		
409	*			475	PRA		As/Bar
410	RCL IND 25			476	PRX	→	
411	*			477	7ISZ		
412	2			478	RCL IND 25		
413	/						
414	STO 22			479	LBL 28		Divides by 12
415	RCL IND 25			480	12		
416	*			481	/		
417	3			482	RTN		
418	/						
419	STO 23		$M_A = \frac{VL}{3}$	483	LBL 29		
420	RCL 04		Rectangular Shear	484	"A S"		
421	RCL 03			485	PRA		
422	-			486	RCL 26		
423	RCL IND 25			487	/		
424	*			488	PRX	→	Print $A_s = \frac{M}{ad}$
425	RCL 22			489	RTN		
426	-						
427	STO 22			490	LBL H		
428	LASTX	→	$\left[\frac{f_p \max - w_s t}{2} \right]$	491	ADV		
429	+		$\times \frac{L-C}{2} = V$	492	RCL 02		
430	RCL IND 25			493	RCL 01		
431	*		$M_{Rectangular} = \frac{VL}{2}$	494	*		
				495	.288		
				496	*		
				497	PRX	→	$A_s = 0.002 \times 144 \times W \times T$

Program Listing

HP-41c

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STEP	KEY ENTRY	KEY CODE	COMMENTS	STEP	KEY ENTRY	KEY CODE	COMMENTS
498	11			567	*		
499	STO 25			568	RCL 06		
500	RCL IND 25			569	+		
501	PRX		Print $M_{max} @ T_o$	570	+		
502	RCL 04			571	RCL 04		
503	X↑2			572	RCL 06		
504	RCL 27			573	+		
505	*			574	*		
506	1000			575	.11		
507	/			576	*		
508	PRX		$M_{allow} = FK$	577	"V ALLOW"		
509	RCL 02		$= \frac{bd^2}{12000} \times K$	578	PRA		
510	RCL 00			579	PRX		Print V_{allow}
511	*			580	ADV		on footing
512	.288			581	ADV		
513	*			582	GTO A		
514	ADV			583	.END.		
515	PRX		$AS = 0.002 \times 144 \times L \times T$				
516	14						
517	STO 25						
518	RCL IND 25						
519	PRX		$M_{max} Transverse$				
520	RCL 06						
521	X↑2						
522	RCL 27						
523	*						
524	1000						
525	/						
526	PRX		$M_{allow} = \frac{bd^2}{12000} \times K$				
527	10						
528	STO 25						
529	RCL 03		Ftg wgt & Soil wgt				
530	RCL IND 25		$= W$				
531	X↑2						
532	*						
533	2						
534	/		$M = WL^2/2$				
535	RCL 04						
536	/						
537	RCL 01						
538	*						
539	"TOP"		Top				
540	PRA						
541	XEQ 29		$As = \frac{M}{ad} \times W$				
542	RCL 01						
543	XEQ 22		No & Area of Bars				
544	13						
545	STO 25						
546	RCL 03						
547	RCL IND 25						
548	X↑2						
549	*						
550	2						
551	/						
552	RCL 06						
553	/						
554	RCL 00						
555	*						
556	XEQ 29		Print $As = \frac{M}{ad}$				
557	RCL 00						
558	XEQ 22		No & Area of bars				
559	ADV						
560	RCL 21						
561	12						
562	*						
563	RCL 04						
564	+						
565	RCL 20						
566	12						

$$b_o = [(B+c) \times 12 + d_{main} + d_{transverse}] \times 2$$

$$V_{allow} = b_o d \times v_c$$

$$v_c = 2\sqrt{f_c} = 0.11 \frac{K}{in}$$

POLE FOUNDATIONS "POLEFDN"

PROGRAM DESCRIPTION

Program calculates length of pole foundation, soil pressures, shear reinforcement for 4, 6, and 8 bar arrangements; anchor bolts for 4 and 8 bolt arrangement.

If you should desire a printout of the prompting statements along with the numerical results, set your printer to the normal mode.

The main program may be addressed directly by the execute key (XEQ) and the program name "POLEFDN". Flag eleven has been set causing the program to initialize immediately upon entry of program cards.

Program automatically returns to Label A for your next pole foundation.

The soil pressures are assumed to load the pole foundation with the parabolic load distribution and is computed utilizing formulas developed by Mr. E. Czerniak, AM.ASCE in his paper "Resistance to Overturning of Single, Short Piles" published in ST-2*, pages 1188-1 through 23. This paper should be referred to for derivation of formulas and the passive pressures to be used in the program for various soils.

The pole height and diameter is only for general information and is not used in program.

The required reinforcement, shears, etc. are computed based upon a square column inscribed within the round foundation. However, the minimum reinforcement is based upon the round concrete section.

The concrete stress used by the program is 3000 PSI; However, the steel stress f_y can be grade 40 ($f_y = 40,000$ PSI) or grade 60 ($f_y = 60,000$ PSI) reinforcement. Select as follows:

Label C: $a = 1.44$		Label D: $a = 1.76$	
$K = 226$	Working	$K = 204$	Working
$f'_c = 3000$ PSI	Stress	$f'_c = 3000$ PSI	Stress
$f_y = 40,000$ PSI	Design	$f_y = 60,000$ PSI	Design

If you fail to select a steel stress, the program will automatically use $f_y = 40,000$ PSI. Both concrete stress and f_y selected will be printed for your information.

$f'_c = 3000$ PSI concrete strength

$f_y = 40,000$ PSI or $60,000$ PSI steel strength

$K = 226$ for $f_y = 40,000$ PSI

204 for $f_y = 60,000$ PSI

$a = f_{sj}/12,000 = 1.44$ for $f_y = 40,000$ PSI

1.76 for $f_y = 60,000$ PSI

$M_{allow} = FK = K \times bd^2/12,000$

H_o = Horizontal Force applied to pole (kips)

B = Height of horizontal force above top of pole foundation (feet)

C = Height of pole foundation above the ground line (feet)

Z = Erosion allowance (feet)

$E = B + C + Z$

D = Footing diameter (feet)

R = Passive Pressure ($K/ft^3/ft$ depth)

M_o = Overturning moment (kip-feet)

L = Length of foundation (feet)

f = Earth pressure (kips/SF)

V = Shear (kips)

v = Shear (kips/in²)

A_s = Area of reinforcement (in²)

A = Area of each bolt required (in²)

*ST-2; Journal of the Structural Division of the American Society of Civil Engineers, Vol. 83, No. ST-2, March, 1957.

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ANYWHERE, U.S.A. 00000

JOB—SAMPLE PROBLEM

SHEET NO.

OF

CALCULATED BY

DATE

CHECKED BY

DATE

SCALE

COMM. NO.

5-2

POLE LOCATION
(PUSH LABEL A)

POLE #45

POLE FOUNDATION
(AUGERED-ROUND)

1 of 2

5

POLE HEIGHT =

FT

POLE DIAMETER =

IN @ BASE

$H_o =$

M_{BASE}

$H_o = 1.0$ K ENTER(R/S)

$B = 15.5$ FT ENTER(R/S) $= \frac{M_{BASE}}{H_o} =$

$C = 1.00$ FT

$Z = 1.00$ FT ; $E = 17.5$ FT

D (DIAMETER FT) = 2.0 FT

R (PASSIVE PRESSURE) = 0.30 $\frac{K}{ft}$

$M_o = H_o \times E = 17.5$ KI

$L^3 - 14.14 \frac{H_o L}{R D} - 18.85 \frac{M_o}{R D} = 0$

L (MIN. INITIAL) = 8.0 FT $L = 9.2$ FT

L (actual) = 10.0 FT ENTER(R/S)

Q (PIVOT POINT) = 6.8966 FT

$f_1 = \frac{1.178 (4 M_o + 3 H_o L)^2}{L^2 D (3 M_o + 2 H_o L)} = 0.8124 \frac{K}{ft}$; f_1 (allow) = 1.0345 $\frac{K}{ft} = R \times \frac{1}{2}$

$f_2 = 9.425 (2 M_o + H_o L) / L^2 D = 2.1206 \frac{K}{ft}$; f_2 (allow) = 3.00 $\frac{K}{ft} = R \times L$

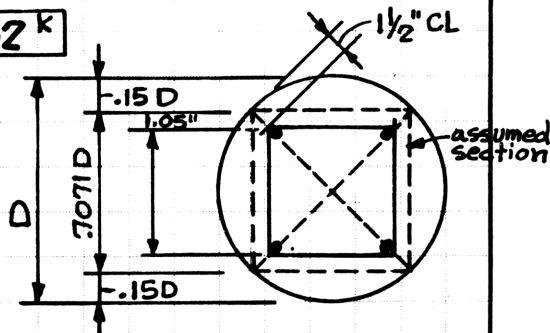
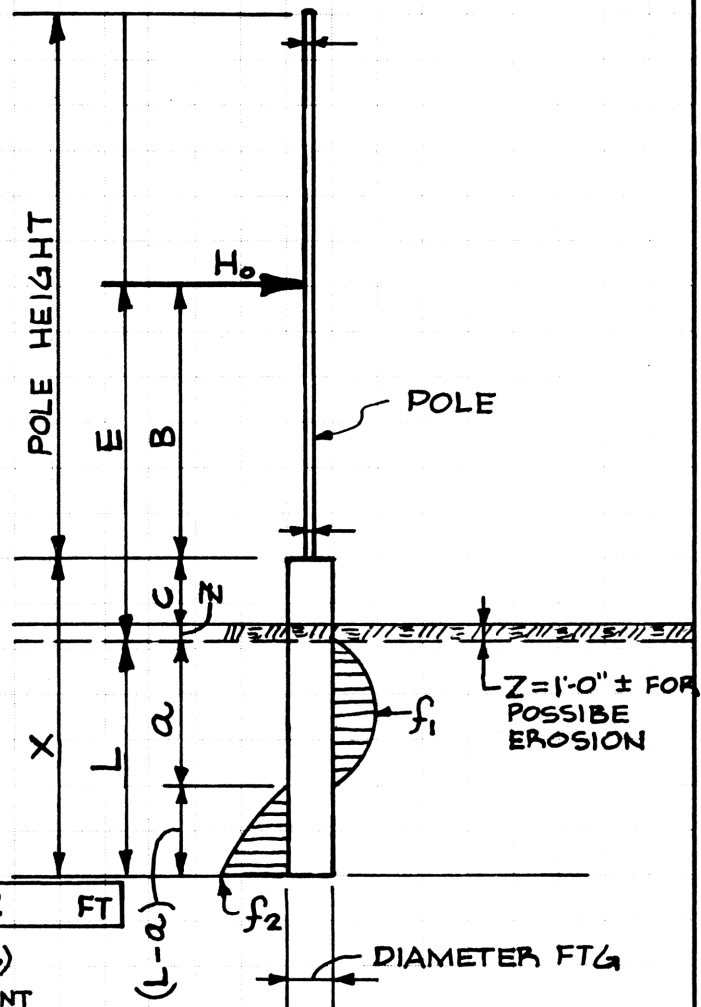
$V(MAX) = H_o \left[1 - 3 \left(\frac{4E}{L} + 3 \right) \left(\frac{Q}{L} \right)^2 + 4 \left(\frac{3E}{L} + 2 \right) \left(\frac{Q}{L} \right)^3 \right] = -3.7562$ K

$U(MAX) = \frac{V(max)}{D^2 \times (2)^2 \times (.7071)^2} = -0.013 \frac{K}{ft^2}$

$M(\frac{1}{2}) = M_o + H_o \frac{Q}{2} - H_o L \left[\left(\frac{4E}{L} + 3 \right) \left(\frac{Q}{2L} \right)^2 + \left(\frac{3E}{L} + 2 \right) \left(\frac{Q}{2L} \right)^3 \right] = 20.9483$ KI

$A_s = \frac{M}{ad} = \frac{M}{1.44d} = 0.9698$ in²

(= $M / 1.76d$)



JOB _____ SAMPLE PROBLEM _____

SHEET NO. _____ OF _____

CALCULATED BY _____ DATE _____

CHECKED BY _____ DATE _____

SCALE _____ COMM. NO. _____

TYPE III

JOB _____
 SHEET NO. _____ OF _____
 CALCULATED BY _____ DATE _____
 CHECKED BY _____ DATE _____
 SCALE _____ COMM. NO. _____

ACI 318-63 "BUILDING CODE REQUIREMENTS
 FOR REINFORCED CONCRETE"

ANCHOR BOLTS

$f'_c = 3000 \text{ PSI}$
 $f_s = 20,000 \text{ PSI}$

$$\text{TENSION DEVELOPMENT LENGTH} = 2 \times \frac{A_s f_s}{\Sigma_o u} \geq \frac{A_s f_s}{\Sigma_o \times 160}$$

$$l = \frac{2 A_s \times 20,000}{\pi D \times \frac{4.8 \sqrt{f'_c}}{D}} = \frac{2 \times 20,000 A_s}{\pi \times 4.8 \sqrt{3000}}$$

$$= \frac{40,000 A_s}{825.95} = 48.4293 A_s$$

$$\text{OR } l = \frac{A_s f_s}{\Sigma_o 160} = \frac{20,000 A_s}{\pi D \times 160} = 39.7887 \frac{A_s}{D}$$

BOLT SIZE	GROSS AREA	ROOT AREA	TENSILE AREA	TENSILE A_s GROSS A_s	DEVELOPMENT LENGTH	
					GROSS AREA	TENSILE STRESS
3/8"φ	.110"	.068"	.078"	.7091	11.68"	8.28"
1/2"φ	.196	.126	.142	.7245	15.60"	11.30"
5/8"φ	.307	.202	.226	.7362	19.55"	14.39"
3/4"φ	.442	.302	.334	.7557	23.45"	17.72"
7/8"φ	.601	.419	.462	.7687	29.11"	22.38"
1"φ	.785	.551	.606	.772	38.02"	29.35"
1 1/8"φ	.994	.693	.763	.7676	48.14"	36.96"
1 1/4"φ	1.227	.890	.969	.7897	59.43"	46.93"
1 3/8"φ	1.485	1.05	1.16	.7811	71.92"	56.18"
1 1/2"φ	1.767	1.29	1.41	.798	85.58"	68.29"

PAGE 5-6

STEP	KEY ENTRY	KEY CODE	COMMENTS	STEP	KEY ENTRY	KEY CODE	COMMENTS
01	*LBL "POLEFDN"		Pole Foundation "POLEFDN"	57	RCL 20		
02	*LBL A			58	*		
03	SF 11			59	RCL 24		$\frac{14.14 H_o}{RD}$
04	SF 12			60	/		
05	"=CALPRO="			61	RCL 23		
06	PRA			62	/		
07	*CALCULATOR PRO"			63	STO 04		
08	"I-GRAMS"			64	ADV		
09	PRA			65	"L MIN"		
10	CF 12			66	PROMPT		L (trial)
11	ADV			67	PRX		
12	"H O"			68	STO 06		
13	PROMPT		Enter Ho				
14	PRX			69	*LBL B		
15	STO 20			70	RCL 06		
16	"B"			71	3		
17	PROMPT		Enter B	72	YTX		
18	PRX			73	RCL 04		
19	STO 00			74	RCL 06		
20	"C"			75	*		
21	PROMPT		Enter C	76	-		
22	PRX			77	RCL 05		
23	STO 01			78	-		
24	"Z"			79	X>0?		
25	PROMPT		Enter Z	80	GTO 16		
26	PRX			81	.1		
27	STO 02			82	ST+ 06		Add 0.1ft to L GTO B if X<0?
28	+			83	GTO B		
29	+						
30	"E"			84	*LBL 16		
31	AVIEW			85	RCL 06		
32	PRX		Print E	86	"L REQD"		
33	STO 03			87	AVIEW		
34	"D"			88	PRX		Print L req'd
35	PROMPT		Enter D	89	"L ACTUAL"		
36	PRX			90	PROMPT		
37	STO 23			91	PRX		Print L(actually used)
38	"R"			92	STO 07		
39	PROMPT		Enter R (Passive Pressure)	93	ADV		
40	PRX			94	RCL 03		
41	STO 24			95	*		
42	RCL 20			96	4		
43	RCL 03			97	*		
44	*			98	3		
45	"A O"			99	RCL 07		
46	AVIEW			100	X↑2		
47	PRX		Print Mo = Ho E	101	*		
48	STO 21			102	+		
49	18.85			103	6		
50	*			104	RCL 03		
51	RCL 24			105	*		
52	/			106	4		
53	RCL 23			107	RCL 07		
54	/			108	*		
55	STO 05			109	+		
56	14.14			110	/		
			$\frac{18.85 M_o}{RD}$				$a = \frac{4EL + 3L^2}{6E + 4L}$

REGISTERS

00	B	01	C	02	Z	03	E	04	14.14 RD	05	18.85 M _o RD	06	L req'd	07	L selected	08	a	09	8.5 D
10		11		12		13		14		15		16		17		18		19	
20	H _o	21	M _o	22		23	D	24	R Passive Pressure	25		26		27		28		29	
30		31		32		33		34		35		36		37		38		39	
40		41		42		43		44		45		46		47		48		49	
50		51		52		53		54		55		56		57		58		59	

STEP	KEY ENTRY	KEY CODE	COMMENTS	STEP	KEY ENTRY	KEY CODE	COMMENTS
111	"a PIVOT"			167	PRX		Print f_2
112	AVIEW			168	RCL 24		
113	PRX		Print a	169	RCL 07		
114	STO 08			170	*		Print $f_{z(max)} = R \times L$
115	3			171	PRX		
116	RCL 20			172	ADV		
117	*			173	1		
118	RCL 07			174	STO 06		
119	*			175	RCL 08		
120	RCL 21			176	RCL 07		
121	4			177	/		
122	*			178	STO 09		
123	+			179	X12		
124	X12			180	3		
125	1.178			181	*		
126	*		$f_1 = \frac{1.178(4M_0 + 3H_0L)^3}{L^2 D(3M_0 + 2H_0L)}$	182	4		
127	RCL 07			183	RCL 03		
128	X12			184	*		
129	/			185	RCL 07		
130	RCL 23			186	/		
131	/			187	3		
132	RCL 20			188	+		
133	RCL 07			189	*		
134	*			190	ST- 06		
135	2			191	RCL 09		
136	*			192	3		
137	3			193	Y1X		
138	RCL 21			194	4		
139	*			195	*		
140	+			196	RCL 03		
141	/			197	3		
142	"F 1"			198	*		
143	AVIEW		Print f_1	199	RCL 07		
144	PRX			200	/		
145	RCL 24			201	2		
146	RCL 08			202	+		
147	*			203	*		
148	2			204	ST+ 06		
149	/			205	RCL 20		
150	PRX		$f_{1(max)} = \frac{R \times a}{2}$	206	ST* 06		
151	RCL 21			207	RCL 06		
152	2			208	"V MAX"		
153	*			209	AVIEW		Print V
154	RCL 20			210	PRX		
155	RCL 07			211	RCL 23		
156	*			212	8.5		
157	+			213	*		
158	9.425		$f_2 = \frac{9.425(2M_0 + H_0L)}{L^2 D}$	214	STO 09		
159	*			215	X12		
160	RCL 07			216	/		
161	X12			217	PRX		Print $v = \frac{V}{bd}$
162	/			218	FC? 00		
163	RCL 23			219	XEQ C		
164	/			220	RCL 21		
165	"F 2"			221	RCL 20		
166	AVIEW			222	RCL 08		

LABELS					FLAGS		SET STATUS							
A	B	C	D	E	0	1	FLAGS		TRIG		DISP			
F	G	H	I	J	2	3	ON OFF		DEG		FIX			
a	b	c	d	e	4	5	0	☐	1	☐	2	☐	3	☐
01	02	03	04	05	6	7	1	☐	2	☐	4	☐	5	☐
06	07	08	09	10	8	9	2	☐	3	☐	6	☐	7	☐
11	12	13	14	15	16	17	3	☐	4	☐	8	☐	9	☐
16	17	18	19	20	24	25	4	☐	5	☐	10	☐	11	☐
21	22	23	24	25			5	☐	6	☐	12	☐	13	☐

STEP	KEY ENTRY	KEY CODE	COMMENTS	STEP	KEY ENTRY	KEY CODE	COMMENTS
223	*			292	RCL 20		
224	2			293	RCL 06		
225	/			294	*		
226	+			295	"M"		
227	"M a/2"			296	AVIEW		
228	AVIEW			297	PRX →		Print M @ Base
229	PRX →		Print M_{max}	298	12		of Pole = $H_o B$
230	RCL 26			299	*		
231	/			300	"BOLT SPACING"		
232	RCL 09			301	PROMPT ←		Enter Bolt
233	2			302	PRX		Spacing
234	-			303	STO 22		
235	STO 23			304	/		
236	/			305	40		
237	PRX →		Print $A_s = \frac{M}{ad}$	306	/		
238	"PAGE 2 OF 2"			307	"AREA BOLT"		
239	AVIEW			308	AVIEW		
240	STO 25			309	PRX →		Print A_s per
241	.005			310	.7071		Bolt L
242	PI			311	/		
243	*			312	PRX →		Print A_s for bolt
244	RCL 09			313	.5		across diagonal
245	.7071			314	*		
246	/			315	PRX →		Print A_s for 8
247	STO 06			316	"Fc"		Bolt group across
248	X↑2			317	AVIEW		diagonal
249	*			318	RCL 28		
250	4			319	PRX →		Print F_c
251	/			320	"FY"		
252	"A S"			321	AVIEW		
253	AVIEW			322	RCL 29		
254	PRX →		$A_{s_{min}} = \frac{0.005 \pi D^2}{4}$	323	PRX →		Print f_y
255	RCL 09			324	ADV		
256	"b"			325	ADV		
257	AVIEW			326	ADV		
258	PRX →		b =	327	ADV		
259	RCL 23			328	ADV		
260	2			329	GTO A		Return to A
261	-						
262	.2121			330	LBL C		
263	*			331	SF 00		$f'_c = 3000 \text{ PSI}$
264	RCL 23			332	1.44		$f_y = 40,000 \text{ PSI}$
265	+			333	STO 26		a = 1.44
266	RCL 23			334	226		K = 226
267	/			335	STO 27		
268	RCL 25			336	3000		$f'_c = 3000 \text{ PSI}$
269	*			337	STO 28		
270	"A S"			338	40000		$f_y = 40,000 \text{ PSI}$
271	AVIEW			339	STO 29		
272	PRX →		Print A_s for	340	RTN		
273	RCL 25		an 4 bar group				
274	3			341	LBL D		
275	/			342	SF 00		
276	PRX →		Print A_s for	343	1.76		a = 1.76
277	RCL 23		an 8 bar group	344	STO 26		
278	"d"		$= A_s/3$	345	204		K = 204
279	AVIEW			346	STO 27		
280	PRX →		Print d	347	3000		$f'_c = 3000 \text{ PSI}$
281	X↑2			348	STO 28		
282	RCL 09			349	60000		$f_y = 60,000 \text{ PSI}$
283	*			350	STO 29		
284	RCL 27			351	GTO A		
285	*			352	.END.		
286	12000						
287	/						
288	"M ALLOW"						
289	AVIEW						
290	PRX →		Print $= \frac{Kbd^2}{12000}$				
291	ADV						

BEARING PLATES/BASE PLATES "PLATES"

PROGRAM DESCRIPTION

Bearing Plates

Program calculates the minimum plate width, bearing stress, "n" and thickness required for a bearing plate of uniform thickness for an allowable steel stress of 27 KSI.

Base Plates

Program calculates the area of base plate required for a concrete bearing stress of 750 PSI, actual bearing stress, the maximum "n" or "m", the plate thickness required for an allowable steel stress of 27 KSI.

Program automatically returns to beginning of program ready for the next bearing plate or base plate.

P_{TL} = Total load (KIPS)

K = Toe of fillet (inches)

L = Length of plates (inches)

F_p = Bearing stress under plate (KSI)

n or m = Moment arm (inches)

t = Plate thickness (inches)

B_f = Flange width (inches)

Depth (D) = Column depth (inches)

$$t = \sqrt{\frac{3F_p n^2}{27}} = \sqrt{\frac{F_p n^2}{9}} \quad \text{or} \quad \sqrt{\frac{F_p m^2}{9}}$$

F_b = 27 KSI for all plates

$n = \frac{W}{2} - k$ (bearing plates)

$m = \frac{W - 0.95D}{2}$ (base plates)

$n = \frac{L - 0.8B_f}{2}$ (base plates)

If you should desire a printout of the prompting statements along with the numerical results, set your printer to the normal mode.

The main program may be addressed directly by the execute key (XEQ) and the program name "PLATES". Flag eleven has been set causing the program to initialize immediately upon entry of program cards.

A trial length and width are printed to aid you in a rapid selection of the plate length and width. This trial length and width is equal to the square root of the required plate area.

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JOB SAMPLE PROBLEM

(6-2)

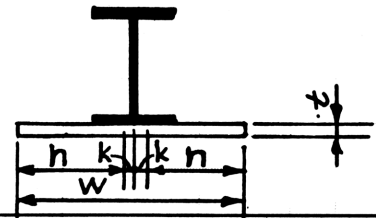
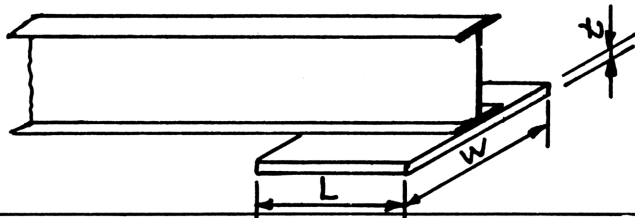
SHEET NO. _____ OF _____

CALCULATED BY _____ DATE _____

CHECKED BY _____ DATE _____

SCALE _____ COMM. NO. _____

PUSH LABEL A		BEARING PLATES									6A
BEAM LOCATION	P_{TL}	BEAM		L (IN)	$F_P(max)$	$W_{(MIN)}$	W_{actual}	$F_{P_{actual}}$	n	t	USE PL
		SIZE	K								
1 A-6	60 ^k	-	1"	8"	0.20 ^k	37.5"	16"	.4688 ^k	7"	1.59 ^k	1 3/4 x 8 x 16
2 B-7	100 ^k	-	1 3/8"	10	0.20	50.0	18	.5556 ^k	7 1/2"	1.89 ^k	2 x 10 x 18
3											
4											
5											
6											
7											
8											
9											
10											
11											
12											
13											
	ENTER (R/S)		ENTER (R/S)	ENTER (R/S)	PRINT	PRINT	ENTER (R/S)	PRINT	PRINT	PRINT	



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JOB **SAMPLE PROBLEM**

6-3

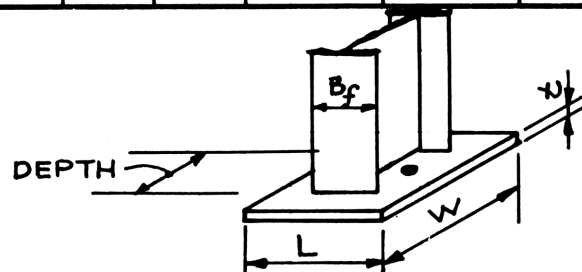
SHEET NO. _____ OF _____

CALCULATED BY _____ DATE _____

CHECKED BY _____ DATE _____

SCALE _____ COMM. NO. _____

PUSH LABEL B		BASE PLATES								6B
COL NO.	"P" (AXIAL LOAD)	AREA (REQ'D)	PLATE SIZE		"F _p "	COLUMN		"N" OR "M"	"t"	USE PLATE
			L	W		"B _f "	DEPTH			
A6	60 ^K	80 ^M	10"	10"	0.60 ^K	8"	8"	1.80"	0.4648"	1/2x10x10
B7	160	213.3	15	15	0.711	8	8	4.30	1.2087	1/4x15x15
H12	260	346.66	18	20	0.7222	8	10	5.80	1.6430	3/4x18x20



STEP	KEY ENTRY	KEY CODE	COMMENTS	STEP	KEY ENTRY	KEY CODE	COMMENTS
	01*LBL "PLATES"		"PLATES"	56 PRA			
	02*LBL 01			57 PRX →			Print n
	03 SF 11			58 X↑2			
	04 SF 12			59 *			
	05 "="CALPRO="			60 9			
	06 PRA			61 /			
	07 "CALCULATOR PRO"			62 SORT			
	08 "FGRAMS"			63 "THICKNESS"			
	09 PRA			64 PRA			Print $t = \sqrt{\frac{F_P n^2}{9}}$
	10 CF 12			65 PRX →			
	11 ADV			66 ADV			
	12 RTN			67 ADV			
				68 GTO A			Return to A
	13*LBL A		Bearing $\#$	69*LBL B			Base $\#$
	14 XEQ 01			70 XEQ 01			
15	"BEARING PL"			71 "BASE PL"			
	16 PRA			72 PRA			
	17 "P TL"		Enter P_{TL}	73 "P TL"			Enter P_{TL}
	18 PROMPT ←			74 PROMPT ←			
	19 PRX			75 PRX			
	20 STO 26			76 STO 26			
	21 "K"		Enter K_{beam}	77 .75			
	22 PROMPT ←			78 /			
	23 STO 20			79 "AREA REQD"			
	24 PRX			80 PRA			Print $A = \frac{P_{TL}}{0.75 \frac{1}{2} \text{in}}$
	25 RCL 26			81 PRX →			
	26 "L"		Enter L // to Beam	82 SORT			
	27 PROMPT ←			83 "TRIAL L OR W"			
	28 PRX			84 PRA			Print $L = \sqrt{A}$
	29 STO 21			85 PRX →			
	30 /			86 "L"			Enter L Length // to Flange
	31 STO 24			87 PROMPT ←			
	32 "F P"		Enter $F_P \text{ max}$	88 PRX			
	33 PROMPT ←			89 STO 21			
	34 PRX			90 "W"			Enter W ⊥ to Flange
	35 STO 22			91 PROMPT ←			
	36 /			92 PRX			
	37 "W <MIN>"			93 STO 23			
	38 PRA		Print W_{min}	94 *			
	39 PRX →			95 1/X			
	40 "W"		Enter W ⊥ to Beam	96 RCL 26			
	41 PROMPT ←			97 *			
	42 PRX			98 "F P"			
	43 STO 23			99 PRA			Print $F_P = \frac{P_{TL}}{L \times W}$
	44 RCL 24			100 PRX →			
	45 /			101 STO 22			
	46 1/X			102 RCL 22			
	47 "F P"			103 RCL 21			
	48 PRA		Print $F_P = \frac{P_{TL}}{L \times W}$	104 "B F"			Enter B_f
	49 PRX →			105 PROMPT ←			
	50 RCL 23			106 PRX			
	51 2			107 STO 24			
	52 /			108 .8			
	53 RCL 20			109 *			
	54 -			110 -			
	55 "N"		$n = \frac{W}{2} - K$				$\frac{L - 0.8 B_f}{2} = n$

REGISTERS									
00	01	02	03	04	05	06	07	08	09
10	11	12	13	14	15	16	17	18	19
20	K	L $\#$	F_P	W $\#$	$\frac{P_{TL}}{B_f}$	Depth	P_{TL}	,	
30	31	32	33	34	35	36	37	38	39
40	41	42	43	44	45	46	47	48	49
50	51	52	53	54	55	56	57	58	59

[illegible]

SIMPLE SPAN BEAM/UNIFORM LOAD
SIMPLE SPAN BEAM/UNIFORMLY VARYING LOAD "ULUVL"
PROGRAM DESCRIPTION

Program calculates moment, reactions, moment of inertia for an L/360 allowable deflection, deflections, and bearing plate sizes using standard formulas.

If you should desire a printout of the prompting statements along with the numerical results, set your printer to the normal mode.

The main program may be addressed directly by the execute key (XEQ) and the program name "ULUVL". Flag eleven has been set causing the program to initialize immediately upon entry of program cards.

For the uniformly varying load the calculated moment at centerline is within 98% of maximum value for a triangular load and is exact for a uniform load. The point of zero shear or maximum moment is also calculated.

w - Load on beam (kips/FT) or width of bearing plate (inches)

L - Span length (feet) or length of bearing plate (inches)

R - Reactions (Kips)

M - Moment (Kip-feet)

I - Moment of inertia (in^4)

X - Point of Zero Shear (feet)

t - Plate thickness (inches)

F_p - Bearing pressure under plate (Kips/in^2)

F_y - 36.0 (Kips/in^2) for bearing plates

The programs and calculation sheets are primarily set up for steel beams but can be adapted to wood beams, concrete beams, basement walls, etc.

After designing a beam or bearing plate, control of program returns to Label A or the simple span beam with a uniform load.

Push Label C - The bearing plate program automatically recalls the left hand reaction so that it does not have to be entered. Then proceed with normal execution as noted on calculation sheet.

Push Label D - The bearing plate program automatically recalls the right hand reaction so that it does not have to be entered. Then proceed with normal execution as noted on the calculation sheet.

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JOB SAMPLE PROBLEM

7-2

SHEET NO. OF

CALCULATED BY DATE

CHECKED BY DATE

SCALE COMM. NO.

BEAM LOCATION: **BEAM H 22 - J 27**
(PUSH LABEL A)

SIMPLE SPAN
BEAM

7A

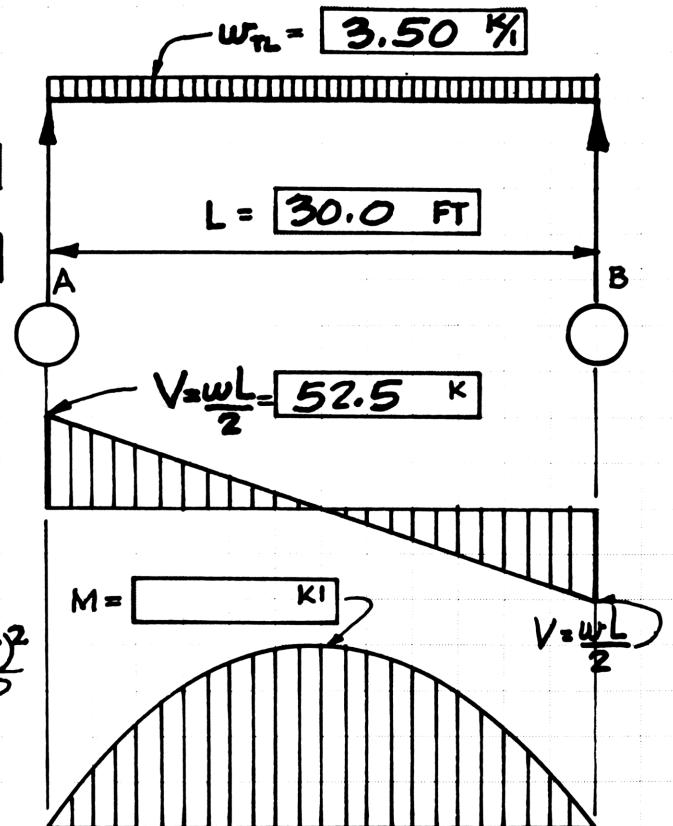
w :

ENTER (R/S)
 $w_{DL} = 2.0 \text{ K/1}$; $w_{LL} = 1.5 \text{ K/1}$
PRINT
 $w_{TL} = 3.5 \text{ K/1}$; $L = 30.0 \text{ FT}$

PRINT
 $R = \frac{wL}{2} = 52.5 \text{ K}$
 $R_{DL} = 30.0 \text{ K}$
 $R_{LL} = 22.5 \text{ K}$

$M = \frac{wL^2}{8} = 393.75 \text{ KI}$

$I(\text{req'd for } \Delta = L/360) = \frac{5 \times 360 w L^3 (12'')^2}{384 \times 29000}$
 $= 0.0232759 w L^3$
 $= 2199.57 \text{ IN}^4$



USE

$M_R = \text{KI}$

$L_c = \text{FT}$

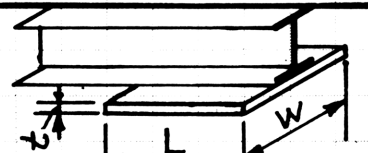
$L_u = \text{FT}$

$I_{xx} = 3000 \text{ IN}^4$ ENTER (R/S)

$\Delta = \frac{5wL^4}{384EI} = \frac{I_{360}}{I_{xx}} \times \frac{L \times 12''}{360}$

$\Delta_{TL} = 1.7332 \text{ IN}$; $\Delta_{LL} = 0.3142 \text{ IN}$

BEARING ϕ
(PUSH LABEL C)



PRINT
 $R_{TL} = 52.5 \text{ K}$; $k_{(beam)} = 1.0 \text{ IN}$

ENTER (R/S)
 $L_{(\phi)} = 8.0 \text{ IN}$; $F_p = 0.2 \text{ K/1}$

PRINT
 $W_{(MIN)} = 32.812 \text{ IN}$; $W_{(\phi)} = 16.0 \text{ IN}$

PRINT
 $F_{p(\phi)} = 0.4102 \text{ K/1}$; $t = 1.4943 \text{ IN}$

USE ϕ **$\phi 1.5 \times 8 \times 16$**

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JOB— SAMPLE PROBLEM

SHEET NO. _____ OF _____

CALCULATED BY _____ DATE _____

CHECKED BY _____ DATE _____

SCALE _____ COMM. NO. _____

7-3

BEAM LOCATION: BEAM B7-D7
(PUSH LABEL B) _____

SIMPLE SPAN BEAM
UNIFORMLY VARYING LOAD

7B

$w_A =$

ENTER (R/S)
 $w_A =$ 0.5 $\frac{K}{Ft}$

$w_B =$

ENTER (R/S)
 $w_B =$ 3.5 $\frac{K}{Ft}$

ENTER (R/S)
 $L =$ 30 FT

PRINT
 $R_A = \frac{L}{6}(2w_A + w_B) =$ 22.5 K

$R_B = \frac{L}{6}(w_A + 2w_B) =$ 37.5 K

$+M_E = \frac{L^2}{16}(w_A + w_B) =$ 225.0 KI *

$I(\text{req'd for } \Delta = \frac{L}{360}) = \frac{5 \times 360 (w_A + w_B) L^3 (12'')^2}{768 \times 29000}$
 $= 0.011637931 (w_A + w_B) L^3$
 $=$ 1256.89 IN⁴ *

BEAM

$M_R =$ _____ KI

$L_C =$ _____ FT

$L_u =$ _____ FT

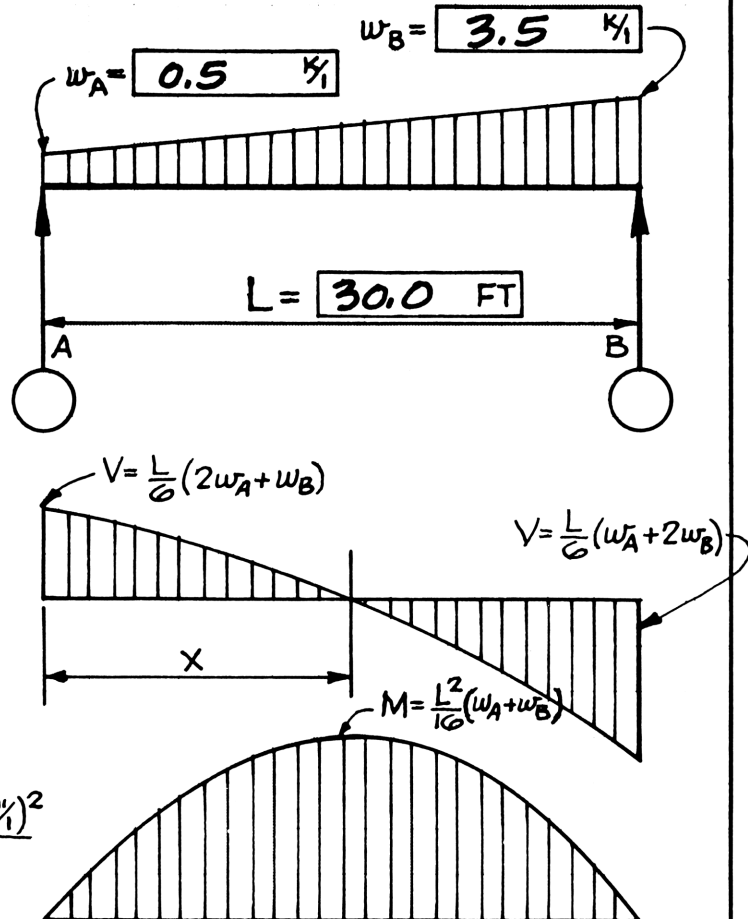
$I_{xx} =$ 1500 IN⁴ ENTER (R/S)

$\Delta_E =$ 0.8379 $= 5 L^4 (w_A + w_B) / 768 EI = \frac{I_{360}}{I_{xx}} \times \frac{L \times 12''}{360} = \frac{I_{360}}{I_{xx}} \times \frac{1}{30}$

BEARING R (PUSH LABEL C FOR R_A ; LABEL D FOR R_B)

PRINT ENTER (R/S) ENTER (R/S) ENTER (R/S) PRINT
 $R_{TL} =$ 22.5 K; $k(\text{beam}) =$ 1.0 IN; $L(\text{R}) =$ 5.0 IN; $F_P(\text{max}) =$ 0.20 $\frac{K}{Ft}$; $W =$ 22.5 IN

$W(\text{PL}) =$ 12.0 IN; $F_P =$ 0.375 $\frac{K}{Ft}$; $t =$ 1.0206 IN; USE $\Phi 1 \frac{1}{8} \times 12 \times 5$



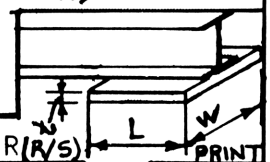
$$\frac{(w_B - w_A)}{2L} X^2 + w_A X - R_A = 0$$

IF $w_A = w_B$ THEN $X = L/2$

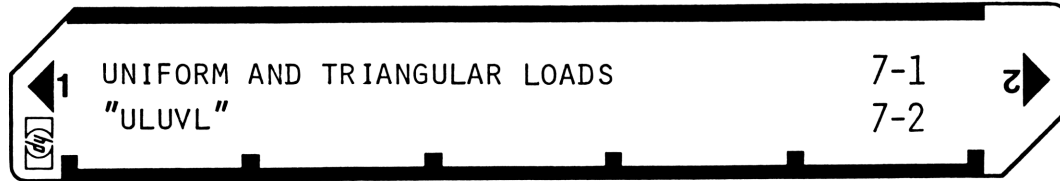
IF $w_A \neq w_B$ THEN

$$X = L \left(\sqrt{\left(\frac{w_A}{w_B - w_A} \right)^2 + \frac{2w_A + w_B}{3(w_B - w_A)}} - \frac{w_A}{w_B - w_A} \right)$$

$X =$ 16.7945 FT



* THESE VALUES ARE WITHIN 98% MAXIMUM.

[illegible]

STEP	KEY ENTRY	KEY CODE	COMMENTS	STEP	KEY ENTRY	KEY CODE	COMMENTS
01	LBL "ULUVL"		"ULUVL"	58	3		
02	LBL A			59	YTX		
03	SF 11			60	*		
04	SF 12			61	"I REQ"		
05	"=CALPRO="			62	PRA		
06	PRA			63	PRX		$I_{x60} = 0.0232759$
07	"CALCULATOR PRO"			64	"I XX"		$\times WL^3$
08	"I GRAMS"			65	PROMPT		Enter I_{xx}
09	PRA			66	PRX		
10	CF 12			67	/		
11	"W-DL"			68	RCL 03		
12	PROMPT		Simple Span Beam	69	*		
13	PRX		Enter w_{DL}	70	30		
14	STO 00			71	/		
15	"W-LL"			72	"DEFLECTION"		
16	PROMPT		Enter w_{LL}	73	PRA		
17	PRX			74	PRX		Print Δ_{TL} max.
18	STO 01			75	RCL 01		
19	+			76	*		
20	ADV			77	RCL 02		
21	PRX		Print w_{TL}	78	/		
22	STO 02			79	PRX		Print Δ_{LL} max
23	"L"			80	ADV		
24	PROMPT		Enter L	81	ADV		
25	PRX			82	GTO A		Return to A
26	STO 03						
27	2			83	LBL B		Uniformly Varying Load
28	/			84	"W-A"		
29	*			85	PROMPT		Enter w_A
30	"REACTIONS"			86	PRX		
31	PRA			87	STO 05		
32	PRX		Print $R_{TL} = \frac{wL}{2}$	88	"W-B"		
33	STO 04			89	PROMPT		Enter w_B
34	RCL 03			90	PRX		
35	2			91	STO 06		
36	/			92	"L"		
37	ENTER↑			93	PROMPT		Enter L
38	ENTER↑			94	PRX		
39	RCL 00			95	STO 07		
40	*			96	RCL 05		
41	PRX		Print R_{PL}	97	2		
42	X<>Y			98	*		
43	RCL 01			99	RCL 06		
44	*			100	"R"		"R"
45	PRX		Print R_{LL}	101	PRA		
46	+			102	+		
47	RCL 03			103	RCL 07		
48	*			104	*		
49	4			105	6		
50	/			106	/		
51	"MOMENT"			107	PRX		Print $R_A = \frac{L}{6}(2w_A + w_B)$
52	PRA			108	STO 04		
53	PRX		Print $M_{TL} = \frac{wL^2}{8}$	109	RCL 05		
54	.0232759			110	RCL 06		
55	RCL 02			111	2		
56	*			112	*		
57	RCL 03			113	+		

REGISTERS									
00	w_{DL}/R	01	w_{LL}	02	w_{TL}	03	L	04	R_{TL}
05	w_A	06	w_B	07	L	08		09	R_B
10		11		12		13		14	
15		16		17		18		19	
20	K	21	$L\phi$	22	F_P	23	$w\phi$	24	
25		26		27		28		29	
30		31		32		33		34	
35		36		37		38		39	
40		41		42		43		44	
45		46		47		48		49	
50		51		52		53		54	
55		56		57		58		59	

STEP	KEY ENTRY	KEY CODE	COMMENTS	STEP	KEY ENTRY	KEY CODE	COMMENTS
114	RCL 07			171	+		
115	*			172	SQRT		
116	6			173	RCL 08		
117	/			174	-		
118	PRX		Print $R_B = \frac{L}{6}(W_A + 2W_B)$	175	RCL 07		
119	STO 09			176	*		
120	RCL 05			177	GTO 14		
121	RCL 06						
122	+			178	LBL 13		$x = L/2$
123	RCL 07			179	RCL 07		
124	X $\uparrow$ 2			180	2		
125	*			181	/		
126	ENTER $\uparrow$						
127	ENTER $\uparrow$			182	LBL 14		
128	16			183	"X"		
129	/			184	PRA		
130	"MOMENT"			185	PRX		Print x (point of zero shear)
131	PRA			186	ADV		
132	PRX		Print $M_E = \frac{L^2}{16}(W_A + W_B)$	187	ADV		
133	X $\langle$ Y			188	GTO A		Return to A
134	RCL 07						
135	*			189	LBL C		Recall R_{ATL}
136	.011637931			190	"R-A"		
137	*			191	RCL 04		
138	"I READ"			192	GTO a		
139	PRA						
140	PRX		Print $I_{\frac{1}{360} \text{ req'd}} = 0.011637931 W L^3$	193	LBL D		Recall R_{BTL}
141	"I XX"			194	"R-B"		
142	PROMPT			195	RCL 09		
143	PRX		Enter I_{xx}				
144	/			196	LBL a		
145	RCL 07			197	PRA		Print R
146	*			198	PRX		
147	30			199	STO 00		
148	/			200	"K"		Enter K_{beam}
149	PRX		Print ΔE	201	PROMPT		
150	RCL 05			202	STO 20		
151	RCL 06			203	PRX		
152	X=Y?			204	RCL 00		
153	GTO 13		set $x = L/2$	205	"L"		Enter L // to Beam
154	RCL 05			206	PROMPT		
155	-			207	PRX		
156	1/X			208	STO 21		
157	*			209	/		
158	STO 00		$x \neq L/2$	210	STO 24		
159	X $\uparrow$ 2			211	"F P"		Enter F_p
160	RCL 05			212	PROMPT		
161	RCL 05			213	PRX		
162	+			214	STO 22		
163	RCL 06			215	/		Print W_{min}
164	+			216	PRX		
165	3			217	"W"		Enter W_{used}
166	/			218	PROMPT		
167	RCL 06			219	PRX		
168	RCL 05			220	STO 23		
169	-			221	RCL 24		
170	/			222	/		

LABELS					FLAGS		SET STATUS							
A	B	C	D	E	0	1	FLAGS		TRIG		DISP			
F	G	H	I	J	2	3	ON OFF		DEG		FIX			
a	b	c	d	e	4	5	0	☐	1	☐	SCI	☐		
01	02	03	04	05	6	7	2	☐	GRAD	☐	ENG	☐		
06	07	08	09	10	8	9	3	☐	RAD	☐	n	4		
11	12	13	14	15	16	17	10	☒	12	☒	13	14	15	
16	17	18	19	20	24	25	18	☐	19	☐	20	21	22	23
21	22	23	24	25			26	☐	27	☐	28	29	30	31

Program Listing

HP-41c

PAGE 7-7

[illegible]

OVERHANGING BEAM "OVERBM"
PROGRAM DESCRIPTION

Program calculates positive and negative moments, inflection points, deflection of interior span and cantilever using standard formulas.

If you should desire a printout of the prompting statements along with the numerical results, set your printer to the normal mode.

The main program may be addressed directly by the execute key (XEQ) and the program name "OVERBM". Flag eleven has been set causing the program to initialize immediately upon entry of program cards.

- w - Uniform load (kips/ft)
- P - Concentrated load (kips)
- L - Span length (feet)
- a - Cantilever length (feet)
- M - Moment (kip-ft)
- R - Reactions (kips)
- Δ - Deflection (inches)

Program and calculation sheet has been primarily set up for a steel beam, but can be easily adapted to wood or concrete beams.

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JOB SAMPLE PROBLEM

SHEET NO. _____ OF _____

CALCULATED BY _____ DATE _____

CHECKED BY _____ DATE _____

SCALE _____ COMM. NO. _____

BEAM LOCATION: **BEAM R12-T14**

(PUSH LABEL A)

OVERHANGING BEAM

8

$P =$

$w =$

$w =$ ENTER (R/S) **2.0** $\frac{\text{K}}{\text{FT}}$

$P =$ ENTER (R/S) **10.0** K

$L =$ ENTER (R/S) **40.0** FT

$a =$ ENTER (R/S) **10.0** FT

$-M_B = Pa + \frac{wa^2}{2} =$ PRINT **200** KI

$R_A = \frac{wL}{2} - \frac{M_B}{L} =$ **35** K

$R_B = \frac{wL}{2} + \frac{M_B}{L} + wa + P =$ **75** K

$+M = (R_A)^2 / 2w =$ **306.25** KI

BEAM

$M_R =$ **_____** KI

$L_c =$ **_____** FT

$L_u =$ **_____** FT

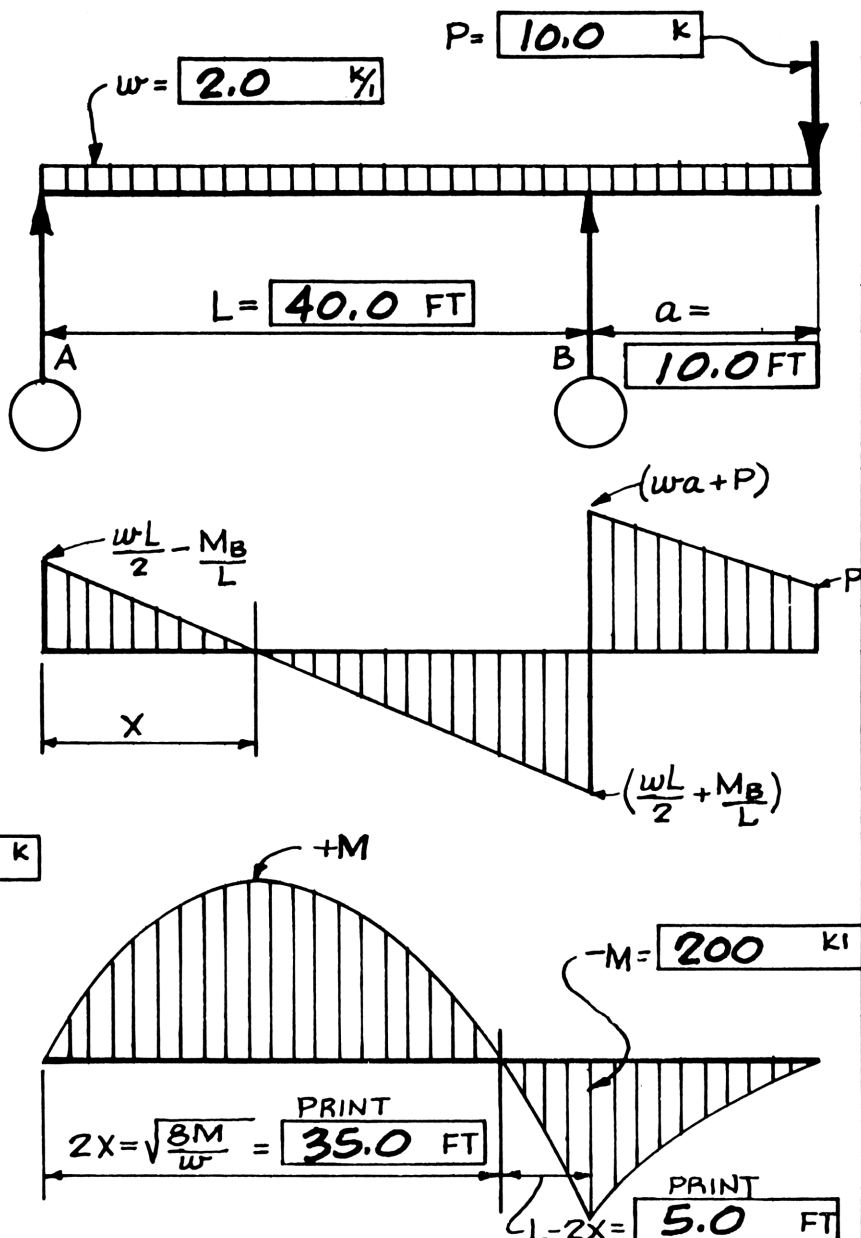
$I_x =$ **1000** IN^4 ENTER (R/S)

$\Delta(\frac{1}{360}) =$ PRINT **1.333** IN

$\Delta(\frac{a}{360}) =$ **0.333** IN

$\Delta_x =$ **2.7739** $\text{IN} = \frac{x}{6EI} \left[\frac{w}{4} (L^4 - 2L^2x^2 + Lx^3 - 2a^2L^2 + 2a^2x^2) - Pa(L^2 - x^2) \right]$

$\Delta_P =$ **-1.2414** $\text{IN} = \frac{wa}{24EI} (4a^2L - L^3 + 3a^3) + \frac{Pa^2}{3EI} (L + a)$



[illegible]

STEP	KEY ENTRY	COMMENTS	STEP	KEY ENTRY	COMMENTS
01	LBL "OVERBM"	Overhanging Beam	58	+	
02	LBL A	"Over bm"	59	RCL 20	
03	SF 11		60	RCL 23	
04	SF 12		61	*	
05	"=CALPRO="		62	+	
06	PRA		63	RCL 21	
07	"CALCULATOR PRO"		64	+	
08	"I-GRAMS"		65	PRX	$R_B = \frac{wL}{2} + \frac{M_B}{L} + wa + P$
09	PRA		66	STO 02	
10	CF 12		67	RCL 01	
11	ADV		68	X<0?	
12	"W K/FT"		69	GTO 15	Skip if uplift all across beam ($R_A \leq 0$)
13	PROMPT	Enter w	70	X+2	
14	PRX		71	2	
15	CLRG		72	/	
16	STO 20		73	RCL 20	
17	"P KIPS"		74	/	
18	PROMPT	Enter P	75	STO 03	$+M = \frac{R_A^2}{2w}$
19	PRX		76	8	
20	STO 21		77	RCL 03	
21	"L"		78	*	
22	PROMPT	Enter L	79	RCL 20	
23	PRX		80	/	
24	STO 22		81	SQRT	$2X = \sqrt{\frac{8M}{w}}$
25	"a FT"		82	STO 04	
26	PROMPT	Enter a	83	RCL 22	
27	PRX		84	-	
28	STO 23		85	CHS	$L - 2X$
29	ADV		86	STO 05	
30	RCL 20				
31	*		87	LBL 15	
32	2		88	RCL 03	
33	/		89	" +M "	
34	RCL 21		90	PRA	Print +M
35	+		91	PRX	
36	RCL 23		92	ADV	
37	*		93	RCL 04	
38	" -M -B "		94	"INFLECTION POIN"	
39	PRA		95	" -T "	
40	PRX	Print $-M_B$	96	PRA	Print 2X
41	STO 00	$= Pa + \frac{wa^2}{2}$	97	PRX	Print L-2X
42	RCL 20		98	RCL 05	
43	RCL 22		99	PRX	
44	*		100	ADV	
45	2		101	" I XX "	Enter I_{xx}
46	/		102	PROMPT	
47	RCL 00		103	PRX	
48	RCL 22		104	STO 24	Deflection
49	/		105	"DEFLECTION"	Prints $L/360$
50	-		106	PRA	
51	"REACTIONS"		107	RCL 22	
52	PRA		108	30	
53	PRX	$R_A = \frac{wL}{2} - \frac{M_B}{L}$	109	/	
54	STO 01		110	PRX	
55	LASTX		111	STO 06	
56	2		112	RCL 23	
57	*		113	30	

REGISTERS

00	-M	01	R_A	02	R_B	03	+M	04	$\frac{2X}{2X \times \frac{12}{2}}$	05	L-2X	06	$L/360$	07	$a/360$	08		09	
10		11		12		13		14		15		16		17		18		19	
20	w	21	P	22	$L \times 12$	23	$a \times 12$	24	I_{xx}	25		26		27		28		29	
30		31		32		33		34		35		36		37		38		39	
40		41		42		43		44		45		46		47		48		49	
50		51		52		53		54		55		56		57		58		59	

STEP	KEY ENTRY	KEY CODE	COMMENTS	STEP	KEY ENTRY	KEY CODE	COMMENTS
114	/			171	*		
115	PRX		Print $a/360$	172	RCL 21		
116	STO 07			173	*		
117	ADV			174	-		
118	RCL 04			175	RCL 04		
119	X=0?			176	*		
120	RCL 22			177	RCL 22		
121	6			178	/		
122	*			179	RCL 24		
123	STO 04		$2X \times \frac{12}{2} = X(in)$	180	/		
124	12			181	174000		
125	ST/ 20		$w/12\%$	182	/		Print Δx
126	ST* 22		$L \times 12\%$	183	PRX		
127	ST* 23		$a \times 12\%$	184	RCL 23		
128	RCL 22			185	X+2		
129	4			186	RCL 22		
130	Y+X			187	*		
131	RCL 22			188	4		
132	X+2			189	*		
133	RCL 04			190	RCL 22		
134	X+2			191	3		
135	*			192	Y+X		
136	2			193	-		
137	*			194	RCL 23		
138	-			195	3		
139	RCL 04			196	Y+X		
140	3			197	3		
141	Y+X			198	*		
142	RCL 22			199	+		
143	*			200	RCL 23		
144	+			201	*		
145	RCL 23			202	RCL 20		
146	X+2			203	*		
147	RCL 22			204	RCL 24		
148	X+2			205	/		
149	*			206	696000		
150	2			207	/		
151	*			208	RCL 22		
152	-			209	RCL 23		
153	RCL 04			210	+		
154	X+2			211	RCL 23		
155	RCL 23			212	X+2		
156	X+2			213	*		
157	*			214	RCL 21		
158	2			215	*		
159	*			216	RCL 24		
160	+			217	/		
161	RCL 20			218	87000		
162	*			219	/		
163	4			220	+		
164	/			221	PRX		Print Δp
165	RCL 22			222	ADV		
166	X+2			223	ADV		
167	RCL 04			224	ADV		
168	X+2			225	ADV		
169	-			226	GTO A		
170	RCL 23			227	JEND.		

$$\Delta x = \frac{X(12\frac{w}{L})^3}{6EI_{xx}} \left[\frac{(w)^4}{4} (L^4 - 2L^2X^2 + LX^3 - 2a^2L^2 + 2a^2X^2) - Pa(L^2 - X^2) \right]$$

$$\Delta p = \frac{wa(12\frac{w}{L})^3}{24EI_{xx}} (4a^2L - L^3 + 3a^3) + \frac{Pa^2(12\frac{w}{L})^3}{3EI_{xx}} (L+a)$$

LABELS					FLAGS		SET STATUS							
A	B	C	D	E	0	1	FLAGS			TRIG		DISP		
F	G	H	I	J	2	3	ON OFF			DEG		FIX		
a	b	c	d	e	4	5	0	☐	☐	GRAD	☐	SCI	☐	
01	02	03	04	05	6	7	1	☐	☐	RAD	☐	ENG	☐	
06	07	08	09	10	8	9	2	☐	☐			n	4	
11	12	13	14	15	16	17	3	☐	☐					
16	17	18	19	20	24	25	10	☒	☐	12	☒	13	14	
21	22	23	24	25			11	☐	☐	18	☐	19	20	
							12	☐	☐	20	☐	21	22	
							13	☐	☐	22	☐	23	24	
							14	☐	☐	24	☐	25	26	
							15	☐	☐	26	☐	27	28	
							16	☐	☐	28	☐	29	30	
							17	☐	☐	30	☐	31	32	

BEAM - UNIFORMLY DISTRIBUTED LOAD "UDLVEM"
AND VARIABLE END MOMENTS
PROGRAM DESCRIPTION

Program calculates maximum positive moment, reactions, inflection points, maximum deflection and $L/360$ deflection limitation using standard formulas.

If you should desire a printout of the prompting statements along with the numerical results, set your printer to the normal mode.

The main program may be addressed directly by the execute key (XEQ) and the program name "UDLVEM". Flag eleven has been set causing the program to initialize immediately upon entry of program cards.

w - Uniform load (Kips/ft), must be greater than zero

M_1 - Moment left side (kip-ft)

M_2 - Moment right side (kip-ft)

M_3 - Positive moment (kip-ft)

L - Span length (feet)

R - Reactions

X - Point of maximum positive moment (feet)

Δ - Deflection (inches)

Program and calculation sheet has been primarily set up for a steel beam, but can be easily adapted to wood or concrete beams.

Bearing Plates

The Bearing Plate program is the same as program number 6A except that labels are used to recall the reactions from storage. After pushing the label recalling the reaction, additional data will be input and printed per program 6A. Labels recall reactions as follows:

LABEL B - Recalls Reaction R_A

LABEL C - Recalls Reaction R_B

If you should enter an improper number for the bearing plate design or accidentally push the wrong label, simply push the proper label and enter your data in the proper sequence.

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JOB SAMPLE PROBLEM
SHEET NO. _____ OF _____
CALCULATED BY _____ DATE _____
CHECKED BY _____ DATE _____
SCALE _____ COMM. NO. _____

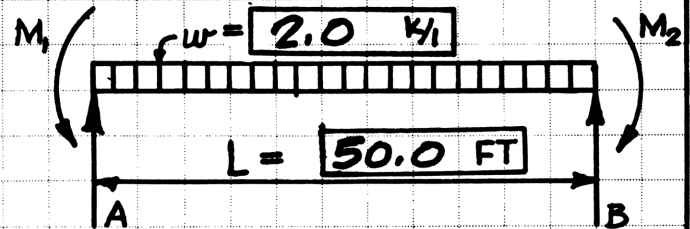
9-2

BEAM LOCATION BEAM A1-A2
(PUSH LABEL A)

BEAM-UNIFORMLY DISTRIBUTED
LOAD AND VARIABLE END MOMENTS

9

w: 2.0 k/ft



w = 2.0 k/ft ENTER(R/S)
(w must be ≠ 0)

-M₁ = 200.0 k ENTER(R/S)

-M₂ = 100.0 k ENTER(R/S)

L = 50.0 FT ENTER(R/S)

R₁ = V₁ = $\frac{wL}{2} + \frac{M_1 - M_2}{L}$ = 52.0 k PRINT

R₂ = V₂ = $\frac{wL}{2} - \frac{M_1 - M_2}{L}$ = 48.0 k

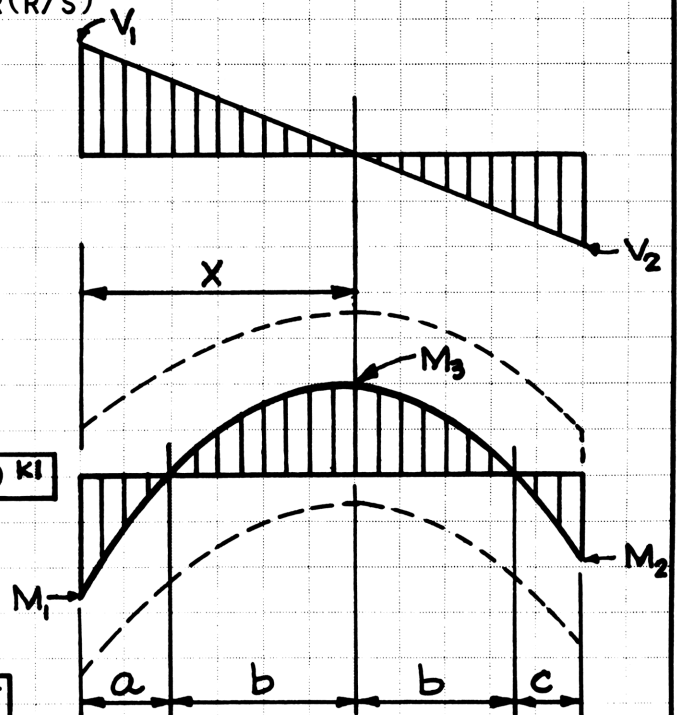
+M₃ = $\frac{wL^2}{8} - \frac{M_1 + M_2}{2} + \frac{(M_1 - M_2)^2}{2wL^2}$ = 476.0 k

X = R₁/w = 26.0 FT

a = x - b = 4.182 FT

b = $\sqrt{M_3 \times \frac{8}{w \times 4}}$ = $\sqrt{\frac{2M_3}{w}}$ = 21.817 FT

c = L - a - 2b = 2.18 FT



BEAM

M_R = k

L_c = FT

L_u = FT

I_x = 4000 in⁴ ENTER(R/S)

$\Delta_x = \frac{wx}{24EI} \left[x^3 - (2L + \frac{4M_1}{wL} - \frac{4M_2}{wL})x^2 + \frac{12M_1}{w}x + L^3 - \frac{8M_1L}{w} - \frac{4M_2L}{w} \right]$

PRINT
 $\Delta_x =$ 1.7259 in ; $\Delta_{max} = \frac{L \times 12}{360} = \frac{L}{30} =$ 1.6667 in PRINT

Prints only for Positive Moments

[illegible]

STEP	KEY ENTRY	KEY CODE	COMMENTS	STEP	KEY ENTRY	KEY CODE	COMMENTS
01	LBL "UDLVEM"		Uniform Load & Variable End Moments "UDLVEM"	57	PRX →		Print M
02	LBL A			58	STO 04		
03	SF 11			59	RCL 02		
04	SF 12			60	RCL 20		
05	"=CALPRO="			61	/		
06	PRA			62	"X"		
07	"CALCULATOR PRO"			63	PRA		
08	"HGRAMS"			64	PRX →		$X = \frac{R_1}{w}$
09	PRA			65	STO 05		
10	CF 12			66	RCL 04		
11	ADV			67	X<0?		If $+M_3$ is negative skip to B
12	"W K/FT"			68	GTO 11		
13	PROMPT ←		Enter w	69	RCL 05		
14	PRX			70	RCL 04		
15	STO 20			71	2		
16	"M1 KIP-FT"			72	*		
17	PROMPT ←		Enter M ₁	73	RCL 20		
18	PRX			74	/		
19	STO 00			75	SQRT		$b = \sqrt{\frac{2M_3}{w}}$
20	"M2 KIP-FT"			76	STO 07		
21	PROMPT ←		Enter M ₂	77	-		
22	PRX			78	"a"		
23	STO 01			79	PRA		Print a = x - b
24	-			80	PRX →		
25	"L"			81	STO 06		
26	PROMPT ←		Enter L	82	RCL 07		
27	PRX			83	"b"		
28	STO 21			84	PRA		Print $b = \sqrt{\frac{2M_3}{w}}$
29	/			85	PRX →		
30	STO 09		$\frac{M_1 - M_2}{L}$	86	2		
31	RCL 20			87	*		
32	RCL 21			88	+		
33	*			89	RCL 21		
34	2			90	-		
35	/			91	CHS		
36	+			92	"c"		
37	"REACTIONS"			93	PRA		Print c = L - a - 2b
38	PRA			94	PRX →		
39	PRX →		$R_1 = \frac{wL}{2} + \frac{M_1 - M_2}{L}$	95	LBL 11		
40	STO 02			96	RCL 00		
41	RCL 09			97	12		
42	2			98	*		
43	*			99	STO 00		
44	-			100	RCL 01		
45	PRX →		$R_2 = \frac{wL}{2} - \frac{M_1 - M_2}{L}$	101	12		
46	STO 03			102	*		
47	RCL 02			103	STO 01		
48	X12			104	RCL 05		
49	2			105	12		
50	/			106	*		
51	RCL 20			107	STO 05		
52	/			108	RCL 20		
53	RCL 00		$+M_3 = \frac{R_1^2}{2w} - M_1$	109	12		
54	-			110	/		
55	"MOMENT"			111	STO 20		
56	PRA			112	RCL 21		

REGISTERS									
00 $-M_1/R$	01 $-M_2$	02 R_1	03 R_2	04 $+M_3$	05 X	06 a	07 b	08 Δx	09 $\frac{M_1 - M_2}{L}$
10	11	12	13	14	15	16	17	18	19
20 w/k	21 L/L	22 $F_P(m)$	23 w/L	24 P/L	25	26	27	28	29
30	31	32	33	34	35	36	37	38	39
40	41	42	43	44	45	46	47	48	49
50	51	52	53	54	55	56	57	58	59

STEP	KEY ENTRY	KEY CODE	COMMENTS	STEP	KEY ENTRY	KEY CODE	COMMENTS
113	12		$\Delta x = \frac{w^x X (12'')^3}{24 E I_{xx}} \left[X^3 - (2L + \frac{4M_1}{wL} - \frac{4M_2}{wL}) X^2 + \frac{12M_1 X}{w} + L^3 - \frac{8M_1 L}{w} - \frac{4M_2 L}{w} \right]$	169	PROMPT		Enter I_{xx}
114	*			170	PRX		
115	STO 21			171	STO 22		
116	RCL 05			172	/		
117	3			173	ST* 08		
118	Y+X			174	RCL 08		
119	STO 08			175	"DEFLECTION"		
120	RCL 09			176	PRA		
121	4			177	PRX		Print Δx
122	*			178	RCL 21		
123	RCL 20			179	360		
124	/			180	/		Print $\Delta = \frac{\Delta x}{360}$
125	RCL 21			181	PRX		
126	2			182	ADV		
127	*			183	GTO A		
128	+						
129	RCL 05			184	*LBL B		Recall R_1
130	X+2			185	"R 1"		
131	*			186	RCL 02		
132	ST- 08			187	GTO a		
133	RCL 00						
134	RCL 05		188	*LBL C		Recall R_2	
135	*		189	"R 2"			
136	12		190	RCL 03			
137	*						
138	RCL 20		191	*LBL a		Bearing Φ	
139	/		192	PRA		Print R	
140	ST+ 08		193	PRX			
141	RCL 21		194	STO 00			
142	3		195	"K"		Enter K_{beam}	
143	Y+X		196	PROMPT			
144	ST+ 08		197	STO 20			
145	RCL 00		198	PRX			
146	RCL 21		199	RCL 00			
147	*		200	"L"		Enter L_Φ	
148	RCL 20		201	PROMPT		// to beam	
149	/		202	PRX			
150	8		203	STO 21			
151	*		204	/		Φ/L	
152	ST- 08		205	STO 24		Enter F_{pmax}	
153	RCL 01		206	"F P"			
154	RCL 21		207	PROMPT			
155	*		208	PRX			
156	RCL 20		209	STO 22			
157	/		210	/		Print W_{min}	
158	4		211	PRX		$= \frac{R}{L \times F_p}$	
159	*		212	"W"		Enter W_Φ	
160	ST- 08		213	PROMPT		$\perp$ to Beam	
161	RCL 20		214	PRX			
162	RCL 05		215	STO 23			
163	*		216	RCL 24			
164	24		217	/		Print F_p	
165	/		218	1/X		$= \frac{R}{W \times L}$	
166	29000		219	PRX			
167	/		220	RCL 23			
168	"I XX"		221	2			

LABELS					FLAGS		SET STATUS							
A	B	C	D	E	0	1	FLAGS			TRIG			DISP	
✓	✓	✓					ON	OFF		DEG		FIX		
F	G	H	I	J	2	3	0	☐	☒	☐		SCI	☐	
a	b	c	d	e	4	5	1	☐	☒	GRAD	☐	ENG	☐	
01	02	03	04	05	6	7	2	☐	☒	RAD	☐	n	4	
							3	☐	☒					
06	07	08	09	10	8	9	10	☒		12	✓	13	14	15
11	12	13	14	15	16	17	18	☒		19	20	21	22	23
16	17	18	19	20	24	25	26	☒		27	28	29	30	31
21	22	23	24	25										

[illegible]

PLASTIC DESIGN OF CONTINUOUS BEAMS "PLASDES"
PROGRAM DESCRIPTION

Program calculates reactions, plastic moments, inflection points, and the allowable unbraced distances using the AISC "Specification for the Design, Fabrication, and Erection of Structural Steel for Buildings" 1978 Edition and AISC PLASTIC DESIGN IN STEEL, 1959 Edition (Page A6).

If you should desire a printout of the prompting statements along with the numerical results, set your printer to the normal mode.

The main program may be addressed directly by the execute key (XEQ) and the program name "PLASDES". Flag eleven has been set causing the program to initialize immediately upon entry of program cards.

Factor Safety - 1.7

w - Uniform load (Kips/ft)

L - Span length (feet)

R - Reactions (kips)

r_y - Radius of gyration

l_{cr} - Critical unbraced distance

M_p - Plastic moment - 1.7M

Bearing Plates

The Bearing Plate program is the same as program number 6A except that labels are used to recall the reactions from storage. After pushing the label recalling the reaction, additional data will be input and printed per program 6A. Labels recall reactions as follows:

LABEL B - Recalls Reaction R_E

LABEL C - Recalls Reaction R₁

LABEL D - Recalls Reaction R₂

LABEL E - Recalls Reaction R₃ = R₄ = etc.

If you should enter an improper number for the bearing plate design or accidentally push the wrong label, simply push the proper label and enter your data in the proper sequence per prompting statement.

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JOB SAMPLE PROBLEM

10-2

SHEET NO. _____ OF _____

CALCULATED BY _____ DATE _____

CHECKED BY _____ DATE _____

SCALE _____ COMM. NO. _____

BEAM LOCATION: **BEAM E7-E8-E9-E10-E11**
(PUSH LABEL A)

PLASTIC DESIGN
CONTINUOUS BEAMS

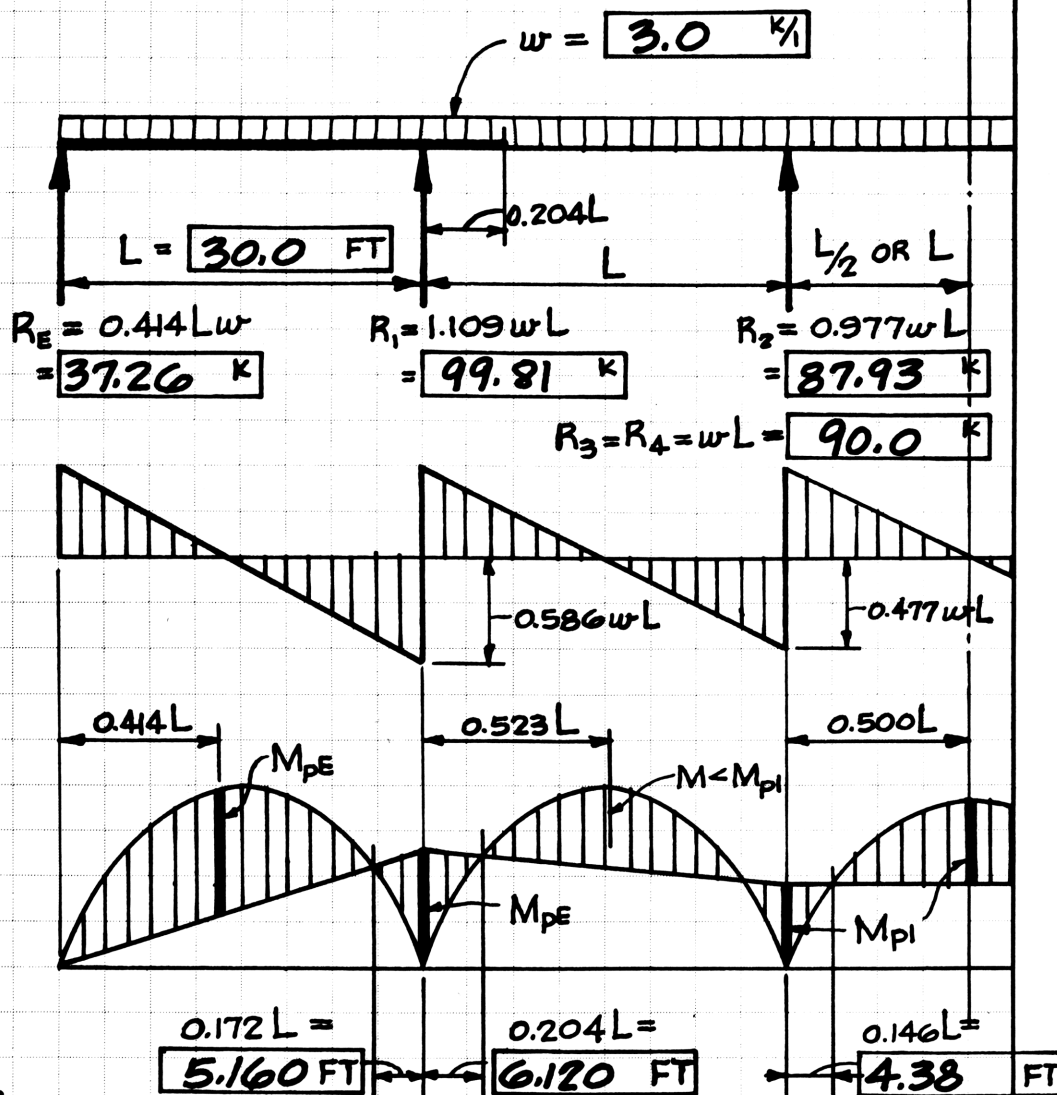
10

w :

SYMM ABOUT $\$$

ENTER (R/S)
 $w = 3.0 \text{ K/1}$

ENTER (R/S)
 $L = 30.0 \text{ FT}$



$M_{pE} = 1.7 \times 0.0858 w L^2$
 $= 393.82 \text{ KI}$ PRINT

$M_{pI} = 1.7 \times w L^2 / 16$
 $= 286.875 \text{ KI}$ PRINT

BEAM

$M_p = \text{KI}$

$r_y = 1.0 \text{ IN}$ ENTER (R/S)

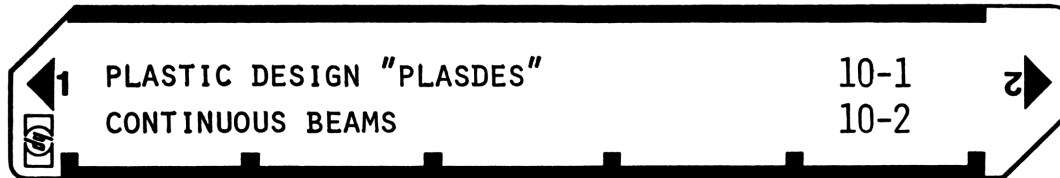
$l_{cr} = \frac{63.2 r_y}{12\%} = 5.2667 \text{ FT}$ PRINT

BEAM

$M_p = \text{KI}$

$r_y = 2.0 \text{ IN}$ ENTER (R/S)

$l_{cr} = \frac{63.2 r_y}{12\%} = 10.5333 \text{ FT}$ PRINT

[illegible]

Program Listing

HP-41c

PAGE 10-4

STEP	KEY ENTRY	KEY CODE	COMMENTS	STEP	KEY ENTRY	KEY CODE	COMMENTS
01	LBL "PLASDES"		Plastic Design Continuous Beams "PLASDES"	56	*		
02	LBL A			57	PRX		Prints 0.146L
03	SF 11			58	"MOMENTS"		Moments
04	SF 12			59	PRA		
05	"=CALPRO="			60	1.7		
06	PRA			61	RCL 20		
07	"CALCULATOR PRO"			62	*		
08	"I-GRAMS"			63	RCL 21		
09	PRA			64	X↑2		
10	CF 12			65	*		
11	ADV		66	STO 06			
12	"W K/FT"		67	.0858			
13	PROMPT		68	*			
14	PRX		69	PRX		$M_{PE} = 0.0858 wL^2 \times 1.7$	
15	STO 20		70	RCL 06			
16	"LENGTH"		71	16			
17	PROMPT		72	/		$M_{PI} = 1.7 \times wL^2 / 16$	
18	PRX		73	PRX			
19	ADV		74	ADV			
20	"REACTIONS"		75	"L CR END"			
21	PRA		76	PRA			
22	STO 21		77	63.2			
23	*		78	"R Y END"			
24	STO 05		79	PROMPT		Enter r_{YE}	
25	.414		80	PRX			
26	*		81	*			
27	PRX		82	12			
28	STO 01		83	/			
29	RCL 05		84	PRX		Print l_{crE}	
30	1.109		85	"L CR INTERIOR"		$= \frac{63.2}{12} r_y$	
31	*		86	PRA			
32	PRX		87	63.2			
33	STO 02		88	"R Y INTERIOR"			
34	RCL 05		89	PROMPT			
35	.977		90	PRX			
36	*		91	*			
37	PRX		92	12			
38	STO 03		93	/			
39	RCL 05		94	PRX		Print	
40	PRX		95	ADV		$l_{cr1} = \frac{63.2}{12} r_y$	
41	STO 04		96	ADV			
42	ADV		97	ADV			
43	"INFLECTION POIN"		98	ADV		Returns to A	
44	"ITS"		99	GTO A			
45	PRA						
46	.172						
47	RCL 21						
48	*						
49	PRX						
50	.204						
51	RCL 21						
52	*						
53	PRX						
54	.146						
55	RCL 21						

REGISTERS									
00 R	01 R_E	02 R_1	03 R_2	04 R_3	05 wL	06 $1.7wL^2$	07	08	09
10	11	12	13	14	15	16	17	18	19
20 w/K	21 L/L_E	22 $F_{P(max)}$	23 w_E	24 P/L	25	26	27	28	29
30	31	32	33	34	35	36	37	38	39
40	41	42	43	44	45	46	47	48	49
50	51	52	53	54	55	56	57	58	59

STEP	KEY ENTRY	KEY CODE	COMMENTS	STEP	KEY ENTRY	KEY CODE	COMMENTS
	110 LBL D		Recalls R ₂	170			
	111 "R 2"						
	112 PRA						
	113 RCL 03						
	114 GTO a						
	115 LBL E		Recalls R ₃				
	116 "R 3"						
	117 PRA						
	118 RCL 04						
	119 LBL a		Bearing θ Print R	180			
	120 PRX →						
	121 STO 00						
	122 "K"		Enter k				
	123 PROMPT ←						
	124 STO 20						
	125 PRX →						
	126 RCL 00						
	127 "LENGTH"		Enter L θ // to Beam				
	128 PROMPT ←						
	129 PRX →			190			
	130 STO 21						
	131 /						
	132 STO 24						
	133 "F P"		Enter F _p				
	134 PROMPT ←						
	135 PRX →						
	136 STO 22						
	137 /						
	138 PRX →		Print W _{min} $= \frac{R}{L F_p}$				
	139 "WIDTH"				200		
	140 PROMPT ←		Enter W θ ⊥ to Beam				
	141 PRX →						
	142 STO 23						
	143 RCL 24						
	144 /						
	145 1/X		Print F _p actual $= \frac{R}{W_L}$				
	146 PRX →				210		
	147 RCL 23						
	148 2		$n = \frac{W \theta}{2} - k$				
	149 /						
	150 RCL 20						
	151 -						
	152 X ^{1/2}						
	153 *						
	154 9						
	155 /		$z \theta = \sqrt{\frac{F_p n^2}{9}}$				
	156 SQRT						
	157 PRX →			220			
	158 ADV						
	159 ADV						
	160 GTO C						
	161 END						

LABELS					FLAGS		SET STATUS							
A ✓	B ✓	C ✓	D ✓	E ✓	0	1	FLAGS		TRIG		DISP			
F	G	H	I	J	2	3	ON OFF		DEG		FIX			
a ✓	b	c	d	e	4	5	0	☐	1	☐	GRAD	☐	SCI	☐
01	02	03	04	05	6	7	2	☐	3	☐	RAD	☐	ENG	☐
06	07	08	09	10	8	9	10	☐	11 ✓	12 ✓	13	14	15	
11	12	13	14	15	16	17	18	☐	19	20	21	22	23	
16	17	18	19	20	24	25	26	☐	27	28	29	30	31	
21	22	23	24	25										

SIMPLE SPAN CONCRETE TEE BEAM "CONCBM"

PROGRAM DESCRIPTION

Program calculates reactions, moment, depth required, slab width, tensile and compressive reinforcements, shear, stirrups and spacing, using ultimate strength design (Load Factor Design). Can be used for slabs, rectangular and tee beams. (Conforms to ACI 318-78)

If you should desire a printout of the prompting statements along with the numerical results, set your printer to the normal mode.

The main program may be addressed directly by the execute key (XEQ) and the program name "CONCBM". Flag eleven has been set causing the program to initialize immediately upon entry of program cards.

Reactions are actual reactions and do not include any load factors.

Regardless of the flange width input, it will be checked to insure that flange width $\leq L/4 \times 12$

$$\leq b_w + 16 H_f$$

The program will print out a required total depth (D) based upon moment requirements, then you must select the total depth (D) that you desire to use and enter it through the R/S key.

The actual and allowable shears are printed so you can visually inspect the result to see if an increase or decrease in depth would be desirable.

If stirrup distance $x = 0$ (zero), then stirrups are not required. Number three (#3) stirrups are used to determine stirrup spacing; if other stirrup sizes are used, multiply s by $\left(\frac{A_s \text{ stirrup}}{0.22} \right)$. You must visually inspect that

$C \leq H_f$. If the flange thickness required is greater than the actual thickness, then you must input a larger flange thickness or seek other means to analyze your beam.

The estimated dead weight of the beam must be input as this program does not calculate the dead weight of the beam.

For a rectangular section, the flange width is same as width of web.

$$\rho \leq \frac{P_b}{2}$$

$$w = 0.20 \quad \text{for } f_y = 40 \text{ KSI THRU } 60 \text{ KSI}$$

$$K_u = \phi f'_c w (1 - 0.59 w) = 0.9 f'_c (0.2) (1 - 0.59 \times 0.2)$$

$$= 0.1588 f'_c$$

$$F = \frac{M_u}{K_u} = \frac{b d^2}{12000}$$

$$d = \sqrt{\frac{12000 M_u}{K_u b_w}} = \sqrt{\frac{12000 M_u}{0.1588 f'_c b_w}} = \sqrt{\frac{75567 M_u}{f'_c b_w}}$$

$$a_u = 0.9 f_y (1 - 0.59 w) / 12000$$

$$= 0.9 f_y (1 - 0.59 (0.2)) / 12000$$

$$= f_y / 15117$$

$$M_u = F K_u = \frac{b d^2}{12000} \times 0.1588 f'_c$$

$$= \frac{b d^2 f'_c}{75567}$$

$$A_s = \frac{M_u}{a_u d} = \frac{15117 M_u}{f_y d} \quad \text{for } M_u \leq K_u F$$

$$A'_s = \frac{12000 M_u}{\phi (d - d') f_y} = \frac{13,333 M_u}{(d - d') f_y}$$

$$A_s = \frac{M_u}{a_u d} + A'_s \quad \text{for } M_u \geq K_u F$$

↑ balanced

$$c/d = 1.18 \frac{w}{\beta_1} = 1.18 \times \frac{0.2}{0.85} = .2776$$

$$c = 0.28 d$$

$$V_u = \frac{w_{TL}}{0.85} (\text{Load Factor}) \left(\frac{L}{2} - \frac{d}{12} - \frac{\text{Support}}{2} \right)$$

$$v_u = \frac{V_u \times 1000}{b_w d} ; \quad v_c = 2 \sqrt{f'_c}$$

$$V'_u = V_u - v_c b d$$

$$X = \frac{V'_u}{w_{TL} \times \text{Load Factor}}$$

$$S = X + 2d ; \quad s = \frac{A_v f_y}{U' b_w} \quad \text{or } s_{(\max)} = d/2$$

f'_c = Concrete strength (PSI)

f_y = Yield strength of reinforcement (PSI)

Load factor = 1.7

b_w = Width of beam web (inches)

H_f = Slab depth (inches)

b = Slab width (inches)

Dl = Distance from bottom of beam to centerline of tensile reinforcement (inches)

w = Uniform load per foot on beam (Kips/Ft)

L = Span length (FT)

R = Reactions (KIPS)

M_u = Ultimate moment (Kip-FT)

d = Effective depth; distance from centroid of tensile reinforcement to extreme fiber of compressive face (inches)

D = Total depth $d + Dl$ (inches)

A_s = Area of tensile reinforcement (in^2)

A'_s = Area of compressive reinforcement (in^2)

V_u = Ultimate shear at distance d (KIPS)

v_u = Ultimate shear stress at distance d (LBS/in^2)

s = Stirrup spacing

S = Distance that stirrups are required out from face of support

$C = 1.18 w/B_1 \quad d = 0.28d$

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JOB SAMPLE PROBLEM

11-4

SHEET NO. _____ OF _____

CALCULATED BY _____ DATE _____

CHECKED BY _____ DATE _____

SCALE _____ COMM. NO. _____

BEAM LOCATION <u>BEAM A7-D7</u>	SIMPLE SPAN CONCRETE BEAM
(PUSH LABEL A)	

TER (R/S)	f'_c	f_y	LOAD FACTOR	BEAM WIDTH = b_w	SLAB DEPTH = H_f	SLAB WIDTH = b	Steel clearance D_I
	4000 #/sq"	60,000 #/sq"	1.7	12.0 IN	6.0 IN	100.0 IN	2.5 IN

$W_{DL} = 2.0 \text{ K/ft}$ $W_{LL} = 1.0 \text{ K/ft}$

ENTER (R/S)	PRINT	ENTER (R/S)
W_{DL}	W_{LL}	W_{TL}
2.0 K/ft	1.0 K/ft	3.0 K/ft
$L = 20.0 \text{ FT}$		

PRINT	R_{TL}	R_{DL}	R_{LL}
	30.0 K	20.0 K	10.0 K

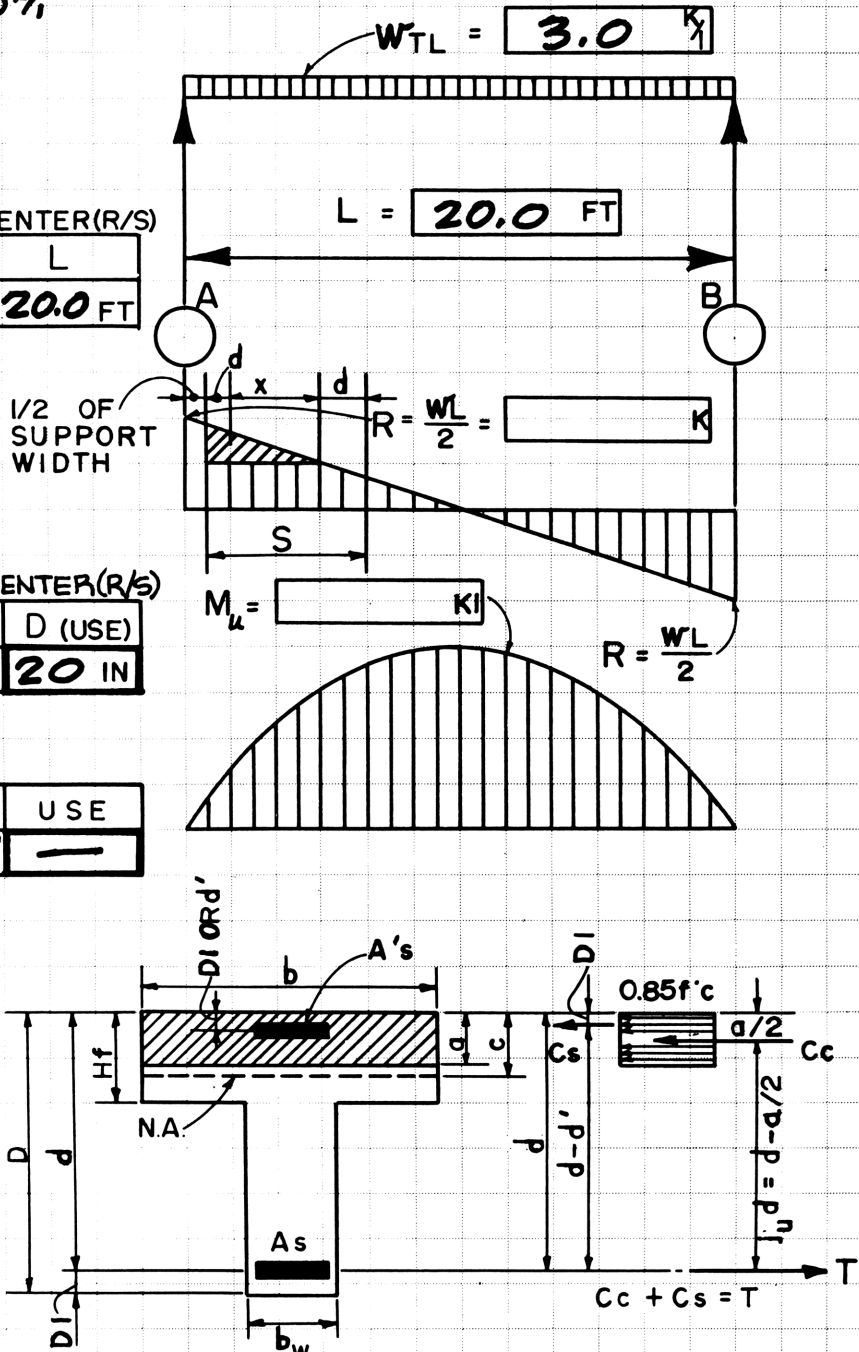
PRINT	ENTER (R/S)
$M_u = 1.7 W_{TL} \frac{L^2}{8}$	MAX SLAB WIDTH
255. KI	60 IN
	D (REQ'D)
	11.4605 IN
	D (USE)
	20 IN

PRINT	$M_u = K_u F$	A_s	USE	A'_s	USE
	972.6 KI	3.67 sq"	4-#9	0.00 sq"	—

ENTER (R/S)	PRINT
1/2 WIDTH OF SUPPORT	$V_u (@ d)$
0.5 FT	48.25 K
	$V_u = \frac{V_u}{b d}$
	229.2 #/sq"

$V_c = 2\sqrt{f'_c}$	$V_u = 8\sqrt{f'_c}$	X
126.49 #/sq"	505.96 #/sq"	4.253 FT

If X=0 stirrups not required
 $A_v = 2 \times 0.11 = 0.22 \text{ sq"}; v \geq 50 \text{ #/sq"}^2$
 S = distance stirrups req'd.



$S = x + 2d$	$s = \frac{A_v f_y}{v' b_w}$	$s = d/2$	$C = 1.18 \frac{W}{b d}$	STIRRUPS
7.169 FT	10.65 IN	8.75 IN	4.9 IN	12 ; #3 @ 8"

$\leq H_f$ Use 12 x 20 Bm w/ 4-#9 B

NO. CARDS 2

[illegible]

STEP	KEY ENTRY	KEY CODE	COMMENTS	STEP	KEY ENTRY	KEY CODE	COMMENTS
001	01+LBL "CONCRN"		Simple Span Concrete Beam	55	2		
	02+LBL A		"CONC BM"	56	/		
	03 SF 11			57	STO 21		
	04 SF 12			58	ADV		
	05 "=CALPRO="			59	"REACTIONS"		
	06 PRA			60	PRA		$R_{TL} = w_{TL} L/2$
	07 "CALCULATOR PRO"			61	PRX		
	08 "FGRAMS"			62	RCL 07		
	09 PRA			63	RCL 09		
	10 CF 12			64	/		
	11 ADV			65	*		
	12 0.02001		Count	66	PRX		R_{DL}
	13 STO 29			67	STO 13		
	14 "F/c"			68	STO 15		
	15 XEQ 15		Enter f'_c	69	RCL 21		
	16 "FY"			70	-		
	17 XEQ 15		Enter f_y	71	CHS		
	18 "LOAD FACTOR"			72	PRX		R_{LL}
	19 XEQ 15		Enter Load Factor	73	STO 14		
	20 STO 12		DL	74	RCL 02		
	21 1			75	*		
	22 ST- 29			76	STO 16		
	23 XEQ 15		$L \cdot F_{LL}$	77	RCL 12		
	24 "BM WIDTH"		Beam Width b_w	78	ST* 15		
	25 XEQ 15			79	RCL 15		
	26 "SLAB DEPTH"		Slab Depth H_f	80	RCL 16		
	27 XEQ 15			81	+		$M_u = (w_{DL} \cdot L \cdot F) \frac{L^2}{8}$
	28 "SLAB WIDTH"		Slab width b_w	82	RCL 20		$+ (w_{LL} \cdot L \cdot F) \frac{L^2}{8}$
	29 XEQ 15			83	*		
	30 "D1"			84	4		
	31 XEQ 15		D1 (steel Clearance)	85	/		
	32 ADV			86	ADV		
	33 "W DL"			87	"M-U"		
	34 XEQ 15		w_{DL}	88	PRA		
	35 "W LL"			89	PRX		Print M_u
	36 XEQ 15		w_{LL}	90	STO 22		
	37 +			91	RCL 05		
	38 "W TL"			92	RCL 20		
	39 PRA			93	4		
	40 STO IND 29			94	/		
	41 ISG 29			95	12		
	42 PRX		w_{TL}	96	*		
	43 "LENGTH"			97	X<=Y?		Flange width
	44 XEQ 15		Length	98	STO 05		$\leq \frac{L}{4} \times 12$
	45 STO 20			99	RCL 05		
	46 GTO 8			100	RCL 04		
	47+LBL 15		Reads, stores	101	16		
	48 PROMPT		and Prints	102	*		
	49 PRX		Data	103	RCL 03		
	50 STO IND 25			104	+		
	51 ISG 29			105	X<=Y?		
	52 RTH			106	STO 05		
	53+LBL 8			107	RCL 22		
	54 *			108	75567		
				109	STO 11		
				110	*		
REGISTERS							
00	f'_c	01	f_y	02	Load factor LL	03	b_w beam width
04	H_f slab Depth	05	b slab Width	06	D1 Steel Centroid	07	w_{DL}
08	w_{LL}	09	w_{TL}	10		11	
12	Load factor DL	13	R_{DL}	14	R_{LL}	15	R_{uPL}
16	R_{uLL}	17		18		19	
20	L	21	R_{TL}	22	M_u	23	d
24	$M_u = F_K$	25	ϕ or M_u	26		27	
28	U'	29	Count	30		31	
32		33		34		35	
36		37		38		39	
40		41		42		43	
44		45		46		47	
48		49		50		51	
52		53		54		55	
56		57		58		59	

STEP	KEY ENTRY	KEY CODE	COMMENTS	STEP	KEY ENTRY	KEY CODE	COMMENTS
	111 RCL 00				165 *		
	112 /				166 RCL 01		
	113 RCL 05				167 /		
	114 "MAX SLAB WIDTH"				168 RCL 23		
	115 PRA				169 RCL 06		
	116 PRX	→	Prints slab width b _w		170 -		
	117 /				171 /		
	118 SQRT				172 +		
	119 RCL 06				173 "AREA STEEL"		
	120 +		$d = \sqrt{\frac{75567 M_u}{f'_c b_w}}$		174 PRA		
	121 "D<REQUIRED"				175 PRX	→	Print A _s
	122 PRA				176 LASTX		
	123 PRX	→	$d_{req'd} = \sqrt{\frac{12000 M_u}{K_u b}}$		177 PRX	→	Print A' _s
	124 "D<USE"		without Compressive reinf		178 RCL 20		
	125 PROMPT	↖	Enter d(used)		179 2		
	126 PRX				180 /		
	127 ADV				181 ADV		
	128 "M-U=K _U -F"				182 "1/2 WIDTH SUPPO"		
	129 PRA				183 "FRT"		
	130 RCL 06				184 PROMPT		
	131 -				185 PRX	→	Enter 1/2 width of support
	132 STO 23				186 "SHEAR"		
	133 X↑2				187 PRA		
	134 RCL 05				188 -		
	135 *				189 RCL 23		
	136 RCL 00				190 12		
	137 *				191 /		
	138 RCL 11				192 -		
	139 /				193 RCL 20		
	140 STO 24				194 2		
	141 PRX	→	$M_u = F K_u$		195 /		
	142 RCL 22		$= \frac{b d^2 f'_c}{75567}$		196 /		
	143 -				197 RCL 15		
	144 X↑0?				198 RCL 16		
	145 0-				199 +		
	146 STO 25				200 .85		
	147 X↑0?				201 /		
	148 GTO 11				202 *		
	149 GTO C				203 PRX	→	$V_u(@d) = w_u L \times L.F. \times (\frac{L}{2} - \frac{d}{12} - \frac{\text{support}}{2}) \times \frac{1}{0.85}$
	150 LBL 11				204 RCL 23		
	151 RCL 22				205 /		
	152 STO 24				206 RCL 03		
	153 LBL C				207 /		
	154 RCL 24				208 3		
	155 RCL 23				209 10↑X		
	156 /				210 *		
	157 RCL 01				211 PRX	→	Print $v_u = \frac{V_u \times 10^3}{b_w d}$
	158 /				212 ADV		
	159 RCL 11				213 "ALLOW SHEAR"		
	160 5				214 "PRA"		
	161 /				215 2		
	162 *				216 RCL 00		
	163 RCL 25				217 SQRT		
	164 13333				218 *		
					219 PRX	→	Print $v_c = 2\sqrt{f'_c}$
					220 4		

LABELS					FLAGS		SET STATUS							
A	B	C	D	E	0	1	FLAGS			TRIG		DISP		
F	G	H	I	J	2	3	ON OFF			DEG	■	FIX	■	
a	b	c	d	e	4	5	0	☐	☐	GRAD	☐	SCI	☐	
01	02	03	04	05	6	7	1	☐	☐	RAD	☐	ENG	☐	
06	07	08	09	10	8	9	2	☐	☐			n	4	
11	12	13	14	15	16	17	3	☐	☐					
16	17	18	19	20	24	25								
21	22	23	24	25										

STEP	KEY ENTRY	KEY CODE	COMMENTS	STEP	KEY ENTRY	KEY CODE	COMMENTS
221	*						
222	PRX →		Prints $V_{u_{max}} = 8\sqrt{f_c}$				
223	4						
224	/						
225	-						
226	STO 28		$V' = V_u - V_c$				
227	RCL 23						
228	*						
229	RCL 03						
230	*						
231	X<0?						
232	0						
233	RCL 07						
234	RCL 12						
235	*						
236	RCL 00						
237	RCL 02						
238	*						
239	+						
240	/						
241	E3						
242	/						
243	"X"						
244	PRA		$X = \frac{V_u}{W_{TL}}$				
245	PRX →						
246	RCL 23						
247	6						
248	/						
249	+						
250	ADV						
251	"STIRRUPS"						
252	PRA		Prints $S = X + \frac{2d}{12}$				
253	PRX →		$V \geq 50 \text{ psi}$				
254	50						
255	RCL 28						
256	X<Y?						
257	X<Y						
258	1/X		$A_v = 0.22 \frac{d}{l}$				
259	.22		(#3 $\square$)				
260	*						
261	RCL 01						
262	*						
263	RCL 03						
264	/						
265	PRX →		$S = \frac{A_v f_y}{V' b_w}$				
266	RCL 23						
267	2						
268	/						
269	PRX →		$S_{max} = d/2$				
270	.56						
271	*						
272	"C"						
273	PRA		$C = 1.18 \frac{w d}{\beta_1}$				
274	PRX →		$= \frac{1.18 (.2) d}{0.85} = 0.28 d$				
275	ADV						
276	ADV						
277	ADV						
278	ADV						
279	GTO A		Returns to A				
280	END.						

SECTION PROPERTIES "SECT"

PROGRAM DESCRIPTION

Program calculates the area, centroid top and bottom, moment of inertia, section modulus top and bottom, radius of gyration, and kern distances top and bottom. The program can handle any number of rectangles, triangles, circles, and known shapes such as angles, wide flange beams, etc. All types of sections can be added or subtracted with intermediate properties calculated at any time without affecting additional sections to be added. Ideal for built-up sections, composite construction, or sections with holes, etc.

If you should desire a printout of the prompting statements along with numerical results, set your printer to the normal mode.

The main program may be addressed directly by the execute key (XEQ) and the program name "SECT". Flag eleven (11) has set causing the program to initialize immediately upon entry of program cards.

LABEL a - Zeros all registers used to add or subtract properties of each element.

1. LABEL A - Prints "RECTANGLE"; rectangular sections, enter B, D, and Y.
To subtract a section enter a negative B.
2. LABEL B - Prints "TRIANGLE"; triangular sections, enter B, D, and Y.
To subtract a section enter a negative B.
3. LABEL C - Prints "ROUND"; round sections, enter RADIUS and Y.
To subtract a section enter a negative RADIUS.
4. LABEL D - Prints "SPECIAL SECTION"; special sections with known properties about their own axis, enter Y, AREA, and I_g. To subtract a section enter a negative AREA and a negative I_g.

FORMULAS:

$$A = B \times D \text{ (Rectangles)}$$

$$M = A \times Y$$

$$I_y = A \times Y^2 = MY$$

$$I_g = \frac{BD^3}{12} \text{ (Rectangles)}$$

$$= \frac{BD^3}{36} \text{ (Triangles)}$$

$$= \frac{R^4 \pi}{4} \text{ (Circles)}$$

$$N_b = \frac{M}{A}$$

$$N_t = \text{Total Depth} - N_b$$

$$I_n = I_y + I_g - \frac{M^2}{A}$$

$$S_t = \frac{I_n}{N_t}$$

$$A = BD/2 \text{ (Triangles)}$$

$$A = \pi R^2 \text{ (Circles)}$$

$$S_b = \frac{I_n}{N_b}$$

$$r = \sqrt{I_n/A}$$

$$K_t = \frac{I_n}{AN_b}$$

$$K_b = \frac{I_n}{AN_t}$$

Program labels each section with a heading to insure that you are computing section properties for the proper section.

Any number of sections can be added or subtracted with intermediate properties calculated at any point. For intermediate properties, you must enter a total depth of only those sections included up to that point.

The base line for determining the Y-distance to neutral axis is the lowest point on the total shape to be input.

A = Area (sq. inches)

B = Width of rectangular or triangular section (inches)

D = Depth of section (inches)

Y = Distance from base line to neutral axis of section to be input (inches)

Radius = Radius of circle (inches)

I_g = Moment of inertia about the centroid of input section (in⁴)

I_n = Moment of inertia about neutral axis (in⁴)

N_b = Neutral axis measured from extreme fiber in bottom of section

N_t = Neutral axis measured from the extreme fiber in top of section

r = Radius of gyration (inches)

K_t = Kern distance above centroid (inches)

K_b = Kern distance below centroid (inches)

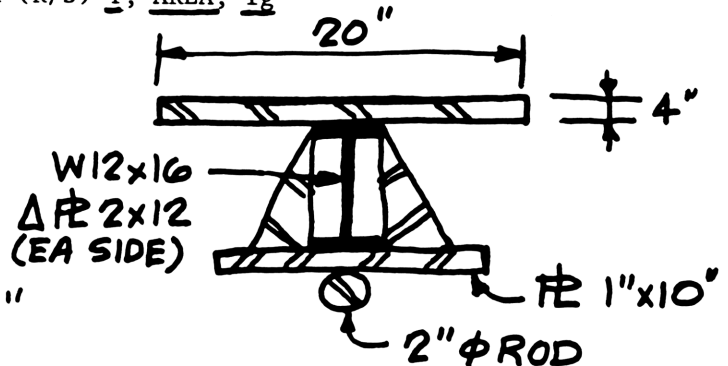
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JOB SAMPLE PROBLEM
SHEET NO _____ OF _____
CALCULATED BY _____ DATE _____
CHECKED BY _____ DATE _____
SCALE _____ **COMM. NO.**

- PUSH LABEL a TO START (ZEROS REGISTERS)
 ① RECTANGULAR SECTIONS--PUSH LABEL A, ENTER (R/S) B; D; Y
 ② TRIANGULAR SECTIONS--PUSH LABEL B, ENTER (R/S) B; D; Y
 ③ ROUND SECTIONS--PUSH LABEL C, ENTER (R/S) RADIUS, Y
 ④ SPECIAL SECTIONS--PUSH LABEL D, ENTER (R/S) Y; AREA; I_g

SKETCH

W12x16
d = 11.99" (Say 12")
A = 4.71
I_g = 103 in⁴
Total depth = 4 + 12 + 1 + 2 = 19"



SECTION PROPERTIES **12**

		ENTER (R/S)	PRINT				
	SECTION	SIZE (B x D)	DISTANCE Y (IN)	A = B · D (IN ²)	M = A · Y (IN ³)	I _y = AY ² = MY (IN ⁴)	I _g = $\frac{BD^3}{12}$ (IN ⁴)
LBL	1	Top Fl	20" x 4"	80	1360	23,120	106.6667
A	2	Bot Fl	10" x 1"	10	25	62.5	0.8333
B	3	Triangles	4" x 12"	24	168	1176	192.0
C	4	Round	1" RADIUS	3.1416	3.1416	3.1416	0.7854
D	5	Beam	W12x16	4.71	42.39	381.51	103.0
	⑤	(PUSH LABEL E)	TOTAL	121.85 in ²	1598.5 in ³	24743.1 in ⁴	403.29 in ⁴

$$N_b = M/A = \boxed{13.1187 \text{ IN}} \quad \text{TOTAL DEPTH} = \boxed{19 \text{ IN}} \quad \text{ENTER R/S}$$

$$N_t = \text{TOTAL DEPTH} - N_b = \boxed{5.8813 \text{ IN}}$$

$$I_n = I_y + I_g - M^2/A = \boxed{4175.8 \text{ IN}^4}$$

$$S_t = \frac{I_n}{N_t} = \boxed{710.01 \text{ IN}^3} \quad S_b = \frac{I_n}{N_b} = \boxed{318.31 \text{ IN}^3}$$

$$r = \sqrt{\frac{I_n}{A}} = \boxed{5.854 \text{ IN}}$$

$$K_t = \frac{I_n}{AN_b} = \boxed{2.612 \text{ IN}}$$

$$K_b = \frac{I_n}{AN_t} = \boxed{5.826 \text{ IN}}$$

- To subtract a section simply put in a negative B (or a negative Area and negative I_g), or a negative RADIUS.
- You can calculate Section Properties at any time by pushing E. It does not affect the registers for adding or deleting a section.

A diagram of a tape with a label. The label contains the text "SECTION PROPERTIES" and "12-1". The tape is shown with a black border and a white background. The label is a black rectangle with white text. The text "SECTION PROPERTIES" is on the top line, and "12-1" is on the bottom line. The tape is shown with a black border and a white background. The label is a black rectangle with white text. The text "SECTION PROPERTIES" is on the top line, and "12-1" is on the bottom line.

[illegible]

STEP	KEY ENTRY	KEY CODE	COMMENTS	STEP	KEY ENTRY	KEY CODE	COMMENTS
01	LBL "SECT"		Program Name "Sect"	56	STO 20		
02	LBL a			57	"D"		
03	SF 11			58	PROMPT		Enter D
04	SF 12			59	PRX		
05	"=CALPRO="			60	STO 21		
06	PRA			61	*		
07	"CALCULATOR PRO"			62	2		
08	"-GRAMS"			63	/		
09	PRA			64	ST+ 00		Area = B·D/2
10	CF 12			65	"Y"		
11	ADV			66	PROMPT		Enter Y
12	CLRG		clears Registers	67	PRX		
13	LBL A		Rectangular Sections	68	STO 22		
14	"RECTANGLE"			69	X<>Y		Print Area = $\frac{BD}{2}$
15	PRA			70	PRX		
16	"B"			71	*		
17	PROMPT		Enter B	72	ST+ 01		
18	PRX			73	PRX		M = A·Y
19	STO 20			74	RCL 22		
20	"D"			75	*		
21	PROMPT		Enter D	76	PRX		I _y = AY ²
22	PRX			77	ST+ 02		
23	STO 21			78	RCL 20		
24	*			79	RCL 21		
25	ST+ 00			80	3		
26	"Y"			81	Y↑X		
27	PROMPT		Enter Y	82	*		
28	PRX			83	36		
29	STO 22			84	/		
30	X<>Y		Area = BD	85	ST+ 03		I = $\frac{BD^3}{36}$
31	PRX			86	PRX		
32	*			87	ADV		
33	ST+ 01			88	STOP		
34	PRX		M = A·Y				
35	RCL 22			89	LBL C		Round Sections
36	*			90	"ROUND"		
37	PRX		I _y = AY ²	91	PRA		
38	ST+ 02			92	"RADIUS"		
39	RCL 20			93	PROMPT		Enter Radius
40	RCL 21			94	STO 20		
41	3			95	X<0?		
42	Y↑X			96	SF 00		Print Radius
43	*			97	PRX		
44	12			98	X↑2		
45	/			99	PI		
46	ST+ 03		I _g = $\frac{BD^3}{12}$	100	*		
47	PRX			101	FS? 00		Negative Radius?
48	ADV			102	CHS		
49	STOP			103	ST+ 00		
50	LBL B		Triangular Sections	104	"Y"		
51	"TRIANGLE"			105	PROMPT		Enter Y
52	PRA			106	STO 22		
53	"B"			107	PRX		Y
54	PROMPT		Enter B	108	X<>Y		Area = $\frac{1}{2}BR^2$
55	PRX			109	PRX		
				110	*		
				111	PRX		M = AY

REGISTERS									
00 Area	01 M	02 I _y	03 I _g	04 I _n	05 I _n /A	06	07	08	09
10	11	12	13	14	15	16	17	18	19
20 B/Rad	21 D	22 Y	23 N _b	24 N _T	25	26	27	28	29
30	31	32	33	34	35	36	37	38	39
40	41	42	43	44	45	46	47	48	49
50	51	52	53	54	55	56	57	58	59

STEP KEY ENTRY KEY CODE COMMENTS			STEP KEY ENTRY KEY CODE COMMENTS		
112	ST+ 01		166	ADV	
113	RCL 22		167	RCL 01	
114	*		168	RCL 00	
115	PRX →	$I_y = AY^2$	169	/	
116	ST+ 02		170	PRX →	$N_b = \frac{M}{A}$
117	RCL 20		171	STO 23	
118	4		172	"DEPTH"	
119	Y+X		173	PROMPT ←	Enter Total depth of Section
120	P1		174	PRX	
121	*		175	-	
122	4		176	CHS	
123	/		177	PRX →	$N_T = \text{Total depth} - N_b$
124	FS? 00		178	STO 24	
125	CHS		179	RCL 02	
126	PRX →	$I_g = \frac{\pi R^4}{4}$	180	RCL 03	
127	ST+ 03		181	+	
128	ADV		182	RCL 01	
129	CF 00		183	X+2	
130	STOP		184	RCL 00	
131	LBL D		185	/	
132	"SPECIAL SECTION"	Special Section	186	-	
133	PRA		187	PRX →	$I_n = I_y + I_g - \frac{M^2}{A}$
134	"Y"		188	STO 04	
135	PROMPT ←	Enter Y	189	RCL 24	
136	PPX		190	/	
137	STO 22		191	PRX →	$S_z = \frac{I_n}{N_T}$
138	"AREA"		192	RCL 04	
139	PROMPT ←	Enter Area	193	RCL 23	
140	PRX		194	/	
141	ST+ 00		195	PRX →	$S_b = \frac{I_n}{N_b}$
142	*		196	RCL 04	
143	PRX →	Print $M = A \cdot Y$	197	RCL 00	
144	ST+ 01		198	/	
145	RCL 22		199	STO 05	
146	*		200	SQRT	
147	PRX →	Prints $I_y = AY^2$	201	PRX →	$r = \sqrt{\frac{I_n}{A}}$
148	ST+ 02		202	ADV	
149	"I G"		203	RCL 05	
150	PROMPT ←	Enter I_g	204	RCL 23	
151	PRX		205	/	
152	ST+ 03		206	PRX →	Top Kern $K_T = \frac{I_n}{A \cdot N_b}$
153	ADV		207	RCL 05	
154	STOP		208	RCL 24	
155	LBL E		209	/	
156	"PROPERTIES"	Section Properties	210	PRX →	Bottom Kern $K_b = \frac{I_n}{A \cdot N_T}$
157	PRA		211	ADV	
158	RCL 00		212	ADV	
159	PRX →	ΣA	213	ADV	
160	RCL 01		214	ADV	
161	PRX →	ΣM	215	STOP	
162	RCL 02		216	END.	
163	PRX →	ΣI_y			
164	RCL 03				
165	PRX →	ΣI_g			
LABELS			FLAGS		
A ✓	B ✓	C ✓	D ✓	E ✓	0 ✓ 1
F	G	H	I	J	2 3
a ✓	b	c	d	e	4 5
01	02	03	04	05	6 7
06	07	08	09	10	8 9
11	12	13	14	15	16 17
16	17	18	19	20	24 25
21	22	23	24	25	
			SET STATUS		
			FLAGS	TRIG	DISP
			ON OFF		
			0 ☐ ☒	DEG ☒	FIX ☒
			1 ☐ ☒	GRAD ☐	SCI ☐
			2 ☐ ☒	RAD ☐	ENG ☐
			3 ☐ ☒		n <u>4</u>
10	11 ✓	12 ✓	13	14	15
16	17	18	19	20	21
22	23	24	25	26	27
30	31	32	33	34	35

FINK TRUSS "FINK"

PROGRAM DESCRIPTION

Program calculates reactions, panel point loads, member axial loads, bending moments in loaded members for varying roof slopes and camber required. Ideal for both steel and wood roof trusses.

If you should desire a printout of the prompting statements along with the numerical results, set your printer to the normal mode.

The main program may be addressed directly by the execute key (XEQ) and the program name "FINK". Flag eleven (11) has been set causing the program to initialize immediately upon entry of program cards.

$$\text{Moment (Top Chord)} = WL^2/10$$

$$\text{Moment (Bottom Chord)} = WL^2/8$$

$$\text{Camber} = K_1 \frac{L^3}{H} + K_2 L^2/H$$

$$K_1 = 0.000032$$

$$H = \frac{L}{2} \times \frac{S}{12}$$

$$K_2 = 0.0028$$

$$= \frac{24L}{S} (0.000032L + .0028)$$

Positive loads act down on top of chord, uplift loads must be input as a negative load.

Program uses the method of joints to solve truss axial loads.

L = Truss span (Feet)

S (Slope) = Amount of truss slope in 12 inches (Inches)

SLOPE (Degrees) - Roof slope converted to degrees

Truss Spacing = Centerline to centerline of adjacent trusses (Feet)

W Top Chord = Uniform roof load on top chord (PSF)

W Bot Chord = Uniform ceiling load on bottom chord (PSF)

R_A, R_F = Reactions (LBS)

P_{AB}, P_{EF}, P_{BC}, P_{CD}, P_{DE} = Panel point loads along top chord (LBS)

P_{AJ}, P_{GF}, P_{JH}, P_{HG} = Panel point loads along bottom chord (LBS)

AXIAL LOAD = Axial load in truss members (LBS) (- Compression, + Tension)

Moment = Moments in top and bottom chord (FT-LBS)

Camber = Inches

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JOB SAMPLE PROBLEM

13-2

SHEET NO _____ OF _____

CALCULATED BY _____ DATE _____

CHECKED BY _____ DATE _____

SCALE _____ COMM NO. _____

FINK ROOF TRUSS T-4
(PUSH LABEL A) _____

ENTER (R/S)		PRINT
L	S (SLOPE)	SLOPE (DEGREES)
30.0 FT	3 / 12	14.0362°

ENTER (R/S)		PRINT
TRUSS SPACING	W TOP CHORD	W BOT CHORD
2 FT	50 PSF	20 PSF

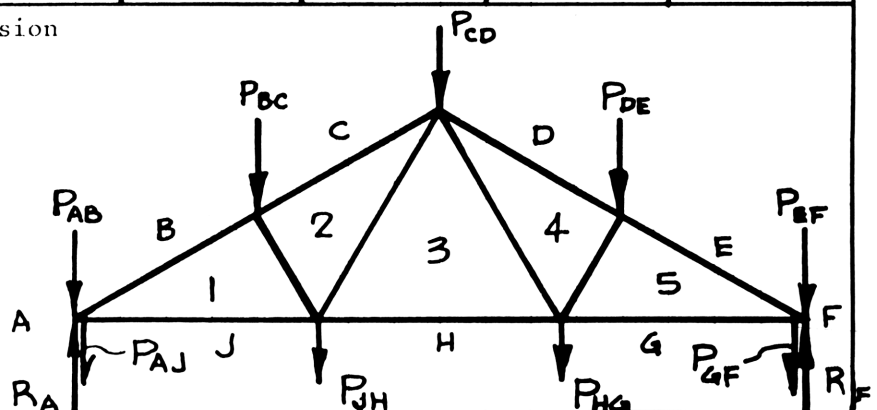
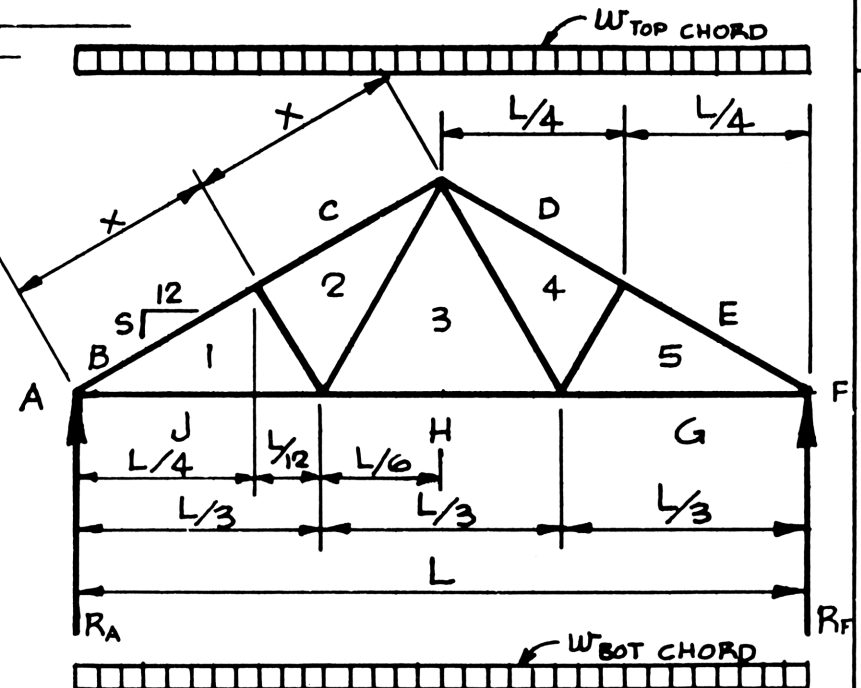
PRINT		PRINT
R _A , R _F	P _{AB} , P _{EF}	P _{BC} , P _{DE} , P _{DE}
2100 LB	375 LB	750 LB

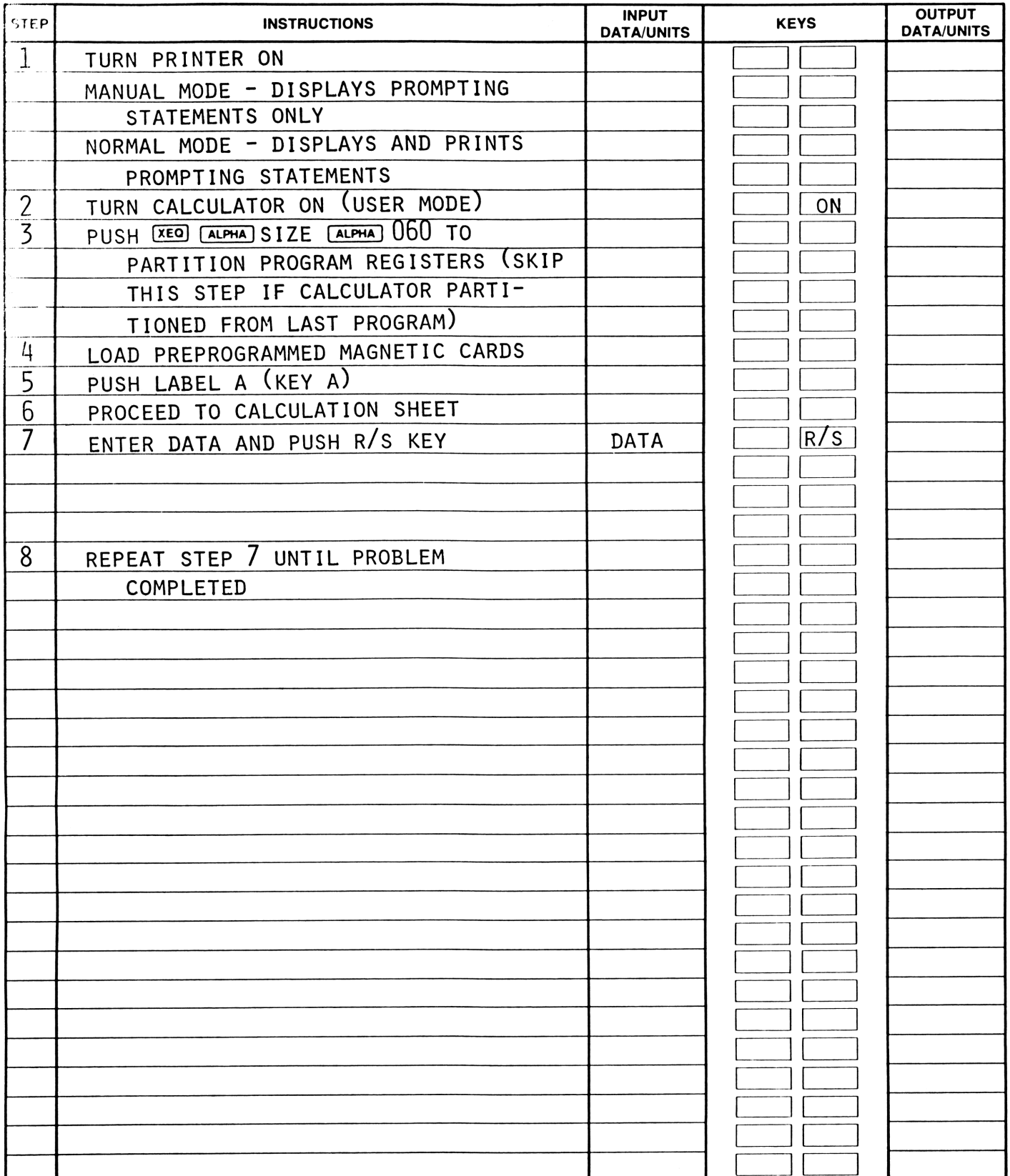
PRINT		PRINT
P _{AJ} , P _{GF}	P _{JH} , P _{HG}	
200 LB	400 LB	

PRINT	TOP CHORD		BOTT CHORD		WEB MEMBERS	
MEMBER	B1, E5	C2, D4	J1, G5	H3	12, 45	23, 34
AXIAL LOAD	-6287.7	-5514.65	+6100	+4066.6	-937.5	+1604.17
MOMENT	562.5	562.5	500	500	—	—

AXIAL LOAD.....LBS { - Compression
MOMENT.....FT LBS { + Tension

CAMBER		
0.9024 IN		





STEP	KEY ENTRY	KEY CODE	COMMENTS	STEP	KEY ENTRY	KEY CODE	COMMENTS												
	01 LBL "FINK"		FINK TRUSS	57	PRX		Print P _{AB}												
	02 LBL A		"FINK"	58	ST- 00														
	03 SF 11			59	2														
	04 SF 12			60	*														
	05 "=CALPRO="			61	PRX		Print P _{BC}												
	06 PRA			62	ADV														
	07 "CALCULATOR PRO"			63	STO 01														
	08 "I-GRAMS"			64	RCL 20														
	09 PRA			65	RCL 22														
	10 CF 12			66	*														
	11 ADV			67	6														
	12 10			68	/														
	13 STO 25			69	RCL 24														
	14 "L"			70	*														
	15 PROMPT	Enter L		71	PRX		Print P _{AJ}												
	16 STO 20			72	ST- 00														
	17 PRX			73	2														
	18 "SLOPE"			74	*														
	19 PROMPT	Enter S (slope)		75	PRX		Print P _{JH}												
	20 STO 21			76	STO 02														
	21 PRX			77	90														
	22 12			78	RCL 00														
	23 R-P			79	XEQ 02														
	24 RDN			80	RCL 21														
	25 PRX	Print Slope in Degrees θ		81	12														
	26 ADV			82	XEQ 03														
	27 "SPACING"			83	0														
	28 PROMPT	Truss Spacing		84	RCL 25														
	29 STO 22			85	XEQ 04														
	30 PRX			86	RCL 10														
	31 "W TOP"			87	RCL 04														
	32 PROMPT	Enter W _{TOP}		88	*														
	33 STO 23			89	STO 08														
	34 PRX			90	RCL 10														
	35 "W BOT"	Enter W _{Bottom}		91	RCL 05														
	36 PROMPT			92	*														
	37 STO 24			93	STO 09														
	38 PRX			94	RCL 01														
	39 ADV			95	ST+ 09														
	40 +			96	RCL 20		$\frac{L}{4} \times 12 \times S$												
	41 *			97	4														
	42 RCL 20			98	/														
	43 *			99	RCL 21		$\frac{L}{12} \times 12$												
	44 2			100	*														
	45 /			101	CHS														
	46 "FORCES"	Forces		102	RCL 20														
	47 PRA			103	XEQ 04														
	48 PRX	Print R _A = $\frac{wL}{2}$		104	STO 27														
	49 STO 00			105	RCL 06														
	50 RCL 20			106	STO 04														
	51 RCL 22			107	*														
	52 *			108	STO 08														
	53 8			109	RCL 27														
	54 /			110	RCL 07														
	55 RCL 23			111	CHS														
	56 *			112	STO 05														
REGISTERS																			
00	-P _{ab}	01	P _{BC}	02	P _{JH}	03	X ₁ Y ₁ - X ₁ Y ₂	04	X ₁	05	Y ₁	06	X ₂	07	Y ₂	08	F _x	09	F _y
10	F _{B1}	11	F _{J1}	12	F _{C2}	13	F _{I2}	14	F _{E23}	15	F _{H3}	16	Top Mchord	17	Bot Mchord	18		19	
20	L	21	S	22	Truss Spacing	23	Top W chord	24	Bot W chord	25	Count	26		27		28		29	
30		31		32		33		34		35		36		37		38		39	
40		41		42		43		44		45		46		47		48		49	
50		51		52		53		54		55		56		57		58		59	

STEP	KEY ENTRY	KEY CODE	COMMENTS	STEP	KEY ENTRY	KEY CODE	COMMENTS
	113 CHS				169 /		
	114 *				170 STO 07		
	115 STO 09				171 PRX →		} M _{Bot} chord = $wL^2/8$ Camber
	116 RCL 02				172 PRX →		
	117 CHS				173 ADV		
	118 ST+ 09				174 "CAMBER"		
	119 RCL 11				175 PRA		
	120 CHS				176 .000032		
	121 ST+ 08				177 RCL 20		
	122 0				178 *		
	123 RCL 25				179 .0028		
	124 XEQ 04				180 +		
	125 ADV				181 RCL 20		$\Delta = \frac{24L}{5} \times$
126	"AXIAL LOAD"		Axial Loads		182 *		$(0.000032L + .0028)$
	127 PRA				183 24		
	128 RCL 10				184 *		
	129 PRX →		Print F _{B1}		185 RCL 21		
	130 RCL 12				186 /		
	131 CHS				187 PRX →		Print Camber
	132 STO 12				188 ADV		
	133 PRX →		Print F _{C2}		189 ADV		
	134 RCL 11				190 ADV		
	135 PRX →		Print F _{J1}		191 ADV		
	136 RCL 15				192 STOP		
	137 PRX →		Print F _{H3}				
	138 RCL 13				193*LBL 02		Components of Force
	139 CHS				194 P-R		
	140 STO 13				195 STO 08		
	141 PRX →		Print F _{I2}		196 X<>Y		
	142 RCL 14				197 STO 09		
	143 PRX →		Print F ₂₃		198 RTN		
	144 ADV						
	145 "MOMENT"		Moment		199*LBL 03		Vector 1 Directional Cosines
	146 PRA				200 R-P		
	147 RCL 23				201 CLX		
	148 RCL 22				202 1		
	149 *				203 P-R		
	150 RCL 20				204 STO 04		
	151 4				205 X<>Y		
	152 /				206 STO 05		
	153 *12				207 RTN		
	154 *						
	155 10				208*LBL 04		Determine Forces Vector 2 Directional Cosines
	156 /				209 R-P		
	157 STO 06				210 CLX		
	158 PRX →		} M _T = $wL^2/10$		211 1		
	159 PRX →				212 P-R		
	160 RCL 24				213 STO 06		
	161 RCL 22			214 X<>Y			
	162 *			215 STO 07			
	163 RCL 20			216 RCL 06			
	164 3			217 RCL 05			
	165 /			218 *			
	166 *12			219 RCL 04			
	167 *			220 RCL 07			
	168 8			221 *			

LABELS					FLAGS		SET STATUS							
A	B	C	D	E	0	1	FLAGS			TRIG		DISP		
✓							ON	OFF	DEG	GRAD	RAD	FIX	SCI	ENG
F	G	H	I	J	2	3	☐	☐	☐	☐	☐	☐	☐	☐
a	b	c	d	e	4	5	☐	☐	☐	☐	☐	☐	☐	☐
01	02 ✓	03 ✓	04 ✓	05	6	7	☐	☐	☐	☐	☐	☐	☐	☐
06	07	08	09	10	8	9	☐	☐	☐	☒	☒	13	14	15
11	12	13	14	15	16	17	☐	☐	☐	☐	☐	21	22	23
16	17	18	19	20	24	25	☐	☐	☐	☐	☐	29	30	31
21	22	23	24	25			☐	☐	☐	☐	☐			

Program Listing

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[illegible]

WOOD COLUMNS "WOODCOL"
PROGRAM DESCRIPTION

This program computes the wood column size required for a given axial load and biaxial bending moments based on the 1977 Edition of the "National Design Specification for Wood Construction" by the National Forest Products Association. Columns are not considered to be braced about either axis within the column length.

If you should desire a printout of the prompting statements along with the numerical results, set your printer to the normal mode.

The main program may be addressed directly by the execute key (XEQ) and the program name "WOODCOL". Flag eleven (11) has been set causing the program to initialize immediately upon entry of program cards.

Members subjected to both flexure and axial tension are proportioned such that

$$\frac{f_t}{F_t} + \frac{f_{bxx}}{F_b} + \frac{f_{byy}}{F_b} \text{ does not exceed the stress duration factor.}$$

Members subjected to both flexure and axial compression are proportioned such that

$$\frac{f_c}{F_c'} + \frac{f_{bxx}}{F_b - Jf_c} + \frac{f_{byy}}{F_b - Jf_c} \text{ does not exceed the stress duration factor.}$$

$$F_c' = F_c \left[1 - \frac{1}{3} \left(\frac{L/D}{K} \right)^4 \right] \leq F_c \text{ when } L/D < K$$

$$F_c' = \frac{0.30E}{(L/D)^2} \text{ when } L/D \geq K$$

$$J = \frac{(L/D) - 11}{K - 11} \quad 0 \leq J \leq 1$$

$$J = 0 \text{ if } L/D \leq 11$$

$$0 < J < 1.0 \text{ if } 11 < L/D < K$$

$$J = 1.0 \text{ if } L/D \geq K$$

$$K = 0.671 \sqrt{\frac{E}{F_c}}$$

Stress Duration Factors:

Permanent	0.90
Normal	1.00
Snow	1.15
Wind	1.33
Impact	2.00

P---+ Compression
- Tension

$L/D \leq 50$

When the depth of a rectangular bending member exceeds 12 inches, the design value for extreme fiber in bending, F_b , shall be multiplied by the size factor, C_F , as determined by the following formula:

$$C_F = \frac{12^{1/9}}{D}$$

<u>When D equals</u>	<u>C_F equals</u>
13.5 in.	0.987
15.5 in.	0.972
17.5 in.	0.959
19.5 in.	0.947
21.5 in.	0.937
23.5 in.	0.928
25.5 in.	0.920
27.5 in.	0.912

IMPORTANT: When Label B is pushed to hold dimension B, and $L/B \geq 50$ about the weak axis, the calculator will continue to run without stopping. Code requirements cannot be satisfied until dimension B is incremented. The calculator will not print any answers until the L/D and $L/B \leq 50$, $f_c \leq \text{S.D.F.} \times F_c$, $f_{bxx} \leq \text{S.D.F.} \times F_b$, and $f_{byy} \leq \text{S.D.F.} \times F_b$ which are internally satisfied by incrementing B and D dimensions.

Pushing Label A will increment B and D dimensions by 2" each increment. Push Label B to increment member depth only. Actual member dimensions are required.

Unless the wood species changes from one column to the next, wood properties are not required to be input each time. Control returns to Label C ready for your next loading combination and member sizes. (P will be displayed)

F_b = Design value for extreme fiber stress in bending
(PSI) for normal or repetitive member usage

F_t = Design value in tension parallel to grain (PSI)

F_c = Design value in compression parallel to grain (PSI)

E = Modulus of elasticity (PSI)

STRESS DURATION FACTOR = Adjustment factor for length of time of load application

P = Total axial load parallel to grain (LBS) (+ Compression; - Tension)

M_{xx} = Moment about the x-axis (FT-LBS)

M_{yy} = Moment about the y-axis (FT-LBS)

L = Actual length of member (FT) assuming pinned ends

B = Actual width of member (IN)

D = Actual depth of member (IN)

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JOB SAMPLE PROBLEM 14-3
SHEET NO _____ OF _____
CALCULATED BY _____ DATE _____
CHECKED BY _____ DATE _____
SCALE _____ COMM. NO. _____

WOOD COLUMN LOCATION <u>B-7</u>					14
(PUSH LABEL A)					
(PUSH LABEL B TO INCREMENT DEPTH (D) AND HOLD WIDTH B)					
ENTER (R/S)					
F _b	F _T	F _c	E	STRESS DURATION FACTOR	WOOD SPECIES
1400 PSI	825 PSI	975 PSI	1.6 × 10 ⁶ PSI	1.15	#2 Southern Pine

(LOCATION LBL C)

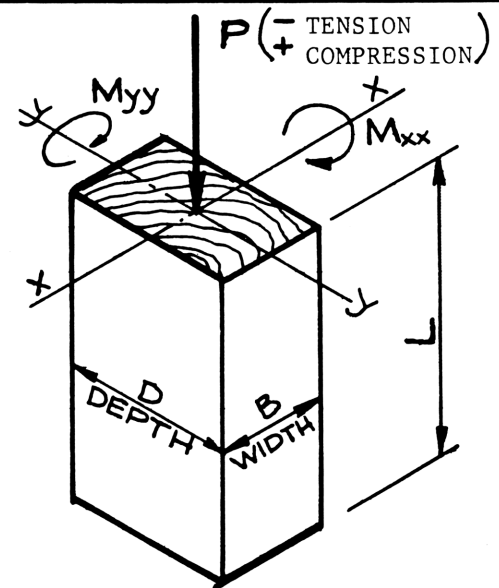
Enter 1.6 →

ENTER (R/S)

P	M _{xx}	M _{yy}
30,000 LBS	400 FT-LBS	300 FT-LBS

ENTER (R/S)

L	B (WIDTH)	D (DEPTH)
15.0 FT	3.5 IN	5.5 IN



PRINT

TRIAL	Tension	f_t/F_t	f_{bxx}/F_b	f_{byy}/F_b	Σ TENSION	MEMBER SIZE
	Comp	f_c/F_c	$f_{bxx}/(F_b - J_y f_c)$	$f_{byy}/(F_b - J_x f_c)$	Σ comp	
1		1.6228	0.1141	0.1415	1.8785	5.5" x 7.5"
2		0.5416	0.0357	0.0381	0.6153	7.5" x 9.5"
3						" x "
4						" x "
5						" x "

- STRESS DURATION FACTOR:
PERMANENT-----0.90 SNOW-----1.15 IMPACT-----2.0
NORMAL-----1.00 WIND-----1.33
- F_b Allowable bending stress for normal or repetitive member usage.
- P + Indicates compression in program
- Indicates tension in program
- M_{xx} and M_{yy} can be positive or negative. They are always converted to a positive moment in program.
- B and D incremented by 2", unless Label B is pressed and then depth only is incremented by 2".
- L/D ≤ 50; L/B ≤ 50
- If the wood properties do not change, enter the next load, etc. (Program returns automatically to Label C ready for next column load).

USE
8x10

STEP	KEY ENTRY	KEY CODE	COMMENTS	STEP	KEY ENTRY	KEY CODE	COMMENTS												
	01*LBL "WOODCOL"		"WOODCOL"	55	12														
	02*LBL A			56	*		Lx12%												
	03 SF 00			57	STO 23														
				58	"B"														
	04*LBL 10			59	PROMPT		Enter B(width)												
	05 SF 11			60	STO 05														
	06 SF 12			61	PRX														
	07 "=CALPRO="			62	"D"														
	08 PRA			63	PROMPT		Enter D (Depth)												
09	"CALCULATOR PRO"			64	STO 06														
	10 "FGRAMS"			65	PRX														
	11 PRA			66	ADV														
	12 CF 12																		
	13 ADV			67*LBL 11															
	14 "Fb"			68	RCL 23														
	15 PROMPT		Enter F _b	69	RCL 05														
	16 PRX			70	RCL 06														
	17 STO 00			71	X<=Y?		B or D smaller												
	18 "F T"			72	X<Y														
	19 PROMPT		Enter F _T	73	RDN														
	20 PRX			74	/		1/B or 1/D Larger												
	21 STO 01			75	STO 24														
	22 "Fc"			76	50														
	23 PROMPT		Enter F _c	77	X<=Y?		1/D ≤ 50												
	24 PRX			78	GTO 01														
	25 STO 02			79	RCL 20														
	26 "E"			80	RCL 05														
	27 PROMPT		Enter E	81	/														
	28 E6			82	RCL 06														
	29 *			83	/														
	30 PRX		E x 10 ⁶ psi	84	STO 07		f _c = P / BD												
	31 STO 03			85	RCL 02														
	32 "S.D.F."			86	RCL 04														
	33 PROMPT		Enter Stress	87	*		f _c ≤ S.D.F. x F _c												
	34 PRX		Duration Factor	88	X<=Y?														
	35 STO 04			89	GTO 01														
				90	RCL 21														
	36*LBL 12			91	72														
	37*LBL C			92	*														
	38 ADV			93	RCL 05		f _{bxx} = 12M _{xx} G / BD ²												
	39 "P"			94	/														
	40 PROMPT		Enter P	95	RCL 06														
	41 PRX			96	X+2														
	42 STO 20			97	/														
	43 "Mxx"			98	STO 08		f _{bxx}												
	44 PROMPT		Enter M _{xx}	99	RCL 06														
	45 PRX			100	RCL 04														
	46 STO 21			101	*														
	47 "Myy"			102	X<=Y?		f _{bxx} ≤ S.D.F. x F _b												
	48 PROMPT		Enter M _{yy}	103	GTO 01														
	49 PRX			104	RCL 22														
	50 STO 22			105	72														
	51 ADV			106	*														
	52 "LENGTH"			107	RCL 06		f _{b_{yy}} = 12M _{yy} G / BD												
	53 PROMPT		Enter L	108	/														
	54 PRX			109	RCL 05														
REGISTERS																			
00	F _b	01	F _T	02	F _c	03	E	04	Stress Duration Factor	05	Col B Width	06	Col D Depth	07	f _c or f _t	08	f _{bxx}	09	Σ
10		11		12		13		14		15		16		17		18		19	
20	P	21	M _{xx}	22	M _{yy}	23	L (inches)	24	1/D	25	f _{b_{yy}}	26		27		28		29	
30		31		32		33		34		35		36		37		38		39	
40		41		42		43		44		45		46		47		48		49	
50		51		52		53		54		55		56		57		58		59	

STEP	KEY ENTRY	KEY CODE	COMMENTS	STEP	KEY ENTRY	KEY CODE	COMMENTS
110	X12			163	/		
111	/			164	4		
112	STO 25		f_{byy}	165	Y+X		$F'_c = F_c \left[1 - \frac{1}{3} \left(\frac{L/D}{K} \right)^4 \right]$
113	RCL 00			166	3		
114	RCL 04			167	/		
115	*			168	CHS		
116	X<Y?		$f_{byy} \leq S.D.F. \times F_b$	169	1		
117	GTO 01			170	+		
118	RCL 07			171	RCL 02		
119	X>0?			172	*		
120	GTO 02			173	GTO 08		
121	ABS		Tension Axial Load	174	LBL 09		
122	RCL 01			175	.3		for $L/D \geq K$
123	/			176	RCL 03		$F'_c = \frac{0.3E}{(L/D)^2}$
124	PRX →		Print f_t/F_T	177	*		
125	RCL 08			178	RCL 24		
126	RCL 00			179	X12		
127	/			180	/		
128	PRX →		Print f_{bxx}/F_b	181	LBL 08		
129	+			182	RCL 07		
130	RCL 25			183	X<Y		
131	RCL 00			184	/		
132	/			185	PRX →		Print f_c/F'_c
133	PRX →		Print f_{byy}/F_b	186	STO 09		
134	+			187	RCL 06		
135	LBL 14			188	XEQ 03		
136	LBL E			189	RCL 07		
137	PRX →		Print Σ	190	*		
138	RCL 05			191	RCL 00		
139	PRX →		Print B (width)	192	X<Y		
140	RCL 06			193	-		
141	PRX →		Print D (Depth)	194	RCL 08		
142	RDN			195	X<Y		
143	RDN			196	/		
144	XEQ 01 →		Check if	197	PRX →		Print $\frac{f_{bxx}}{F_b - J f_c}$
145	ADV		$\frac{f_c}{F_c} + \frac{f_{bxx}}{F_b} + \frac{f_{byy}}{F_b}$	198	ST+ 09		
146	ADV		$\leq$ Stress Duration Factor	199	RCL 05		
147	GTO 12			200	XEQ 03		
148	LBL 01			201	RCL 07		
149	ADV			202	*		
150	RCL 04		Add 2" to depth	203	RCL 00		
151	X>Y?			204	X<Y		
152	RTN			205	-		
153	2		Add 2" to Width	206	RCL 25		
154	ST+ 06		when $F\phi$ is set	207	X<Y		
155	FS? 00			208	/		
156	ST+ 05			209	PRX →		Print $\frac{f_{byy}}{F_b - J f_c}$
157	GTO 11			210	RCL 09		
158	LBL 02			211	+		
159	XEQ 07			212	GTO 14		
160	X>Y?		$L/D > K$	213	LBL 03		
161	GTO 09		$L/D \leq K$	214	RCL 23		
162	X<Y			215	X<Y		

LABELS					FLAGS		SET STATUS								
A ✓	B ✓	C ✓	D	E ✓	0 ✓	1	ON OFF		TRIG		DISP				
F	G	H	I	J	2	3	0	☐	DEG	☒	FIX	☒			
a	b	c	d	e	4	5	1	☐	GRAD	☐	SCI	☐			
01 ✓	02 ✓	03 ✓	04 ✓	05 ✓	6	7	2	☐	RAD	☐	ENG	☐			
06	07 ✓	08 ✓	09 ✓	10 ✓	8	9	3	☐			n	4			
11 ✓	12 ✓	13	14 ✓	15	16	17	10	☒	12 ✓	13	14	15			
16	17	18	19	20	24	25	18	☐	20	21	22	23			
21	22	23	24	25			26	☐	27	28	29	30	31		
							27	☐							

[illegible]

FEET-INCHES-SIXTEENTHS "FT IN16"

PROGRAM DESCRIPTION

Program converts decimal feet to feet-inches-sixteenths, converts feet-inches-sixteenths to decimal feet or simply adds feet-inches-sixteenths as a string of dimensions. You must first enter the number into the display prior to pushing the label key to convert or add your number.

The number in the display will be rounded off to even sixteenths of an inch, but the printed tape will contain the sixteenths and two numbers reflecting fractional sixteenths (not rounded).

1. Label A - Converts decimal feet to FT. IN 16^{ths}
2. Label B - Converts FT. IN 16^{ths} to a decimal number
3. Label C - Clears registers for adding dimensions in Label D and E
Will not clear the display register
4. Label D or Label E - Sums a row of dimensions; all dimensions must be in FT. IN 16^{ths}. You can add a negative dimension provided the total is positive.

	Display	Printed
5. EXAMPLE: Label <u>A</u>	27.6287 Feet 27.07.09 Feet Inches Sixteenths	FT.IN16>DECIMAL 27.6287 *** 27.070871 ***
Label <u>B</u>	35.11 13 Feet Inches Sixteenths 35.9844 Feet	DECIMAL>FT.IN16 35.1113 *** 35.9844 ***
Label <u>D</u> or Label <u>E</u>	35.1113 47.0609 -22.1009 35.1113 Feet Inches Sixteenths 83.0606 Feet 60.07 13 Feet Inches Sixteenths	CLEAR ADD FT.IN16 35.1113 *** 35.111300 *** ADD FT.IN16 47.0609 *** 83.060600 *** ADD FT.IN16 -22.1009 *** 60.071300 ***

Program Listing

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STEP	KEY ENTRY	KEY CODE	COMMENTS	STEP	KEY ENTRY	KEY CODE	COMMENTS		
	01*LBL "FT IN16"		"CALPRO" Calculator Programs	56	INT		Inches 10 ^{ths}		
	02 SF 12			57	STO 06				
	03 "=CALPRO="			58	-				
	04 PRA			59	100				
	05 "CALCULATOR PRO"			60	*				
	06 "+GRAMS"			61	STO 07				
	07 PRA			62	16				
	08 CF 12			63	/				
	09*LBL A		Changes Decimal Number to FT.IN 10 ^{ths} Print Number Feet Feet Inches 10 ^{ths} Prints FT.IN 10 ^{ths}	64	RCL 06		Prints Decimal Number		
	10 ADV			65	+				
	11 STO 01			66	12				
12	"FT.IN16>DECIMAL"			67	/				
	13 PRA			68	RCL 05				
	14 RCL 01			69	+				
	15 PRX			70	PRX				
	16 ENTER↑			71	ADV				
	17 INT			72	RTN				
	18 STO 05								
	19 -								
	20 12								
	21 *								
	22 ENTER↑								
	23 INT								
	24 STO 06								
	25 -								
	26 16								
	27 *								
	28 STO 07								
	29 RCL 05								
	30 RCL 06								
	31 100								
	32 /								
	33 +								
	34 RCL 07								
	35 10000								
	36 /								
	37 +								
	38 FIX 6								
	39 PRX								
	40 FIX 4								
	41 ADV								
	42 RTN								
	43*LBL B		Converts FT.IN10 th To Decimal Feet Prints Number Feet	73*LBL C		Clears Registers 01,02,03,04			
	44 ADV			74 "CLEAR"					
	46 PRA			75 PRA					
	47 PRX			76 0					
	48 ENTER↑			77 STO 01					
	49 STO 01			78 STO 02					
	50 INT			79 STO 03					
	51 STO 05			80 STO 04					
	52 -			81 RDN					
	53 100			82 RTN					
	54 *								
	55 ENTER↑								

[illegible]

